University of Kentucky®

Procurement Services

REQUEST FOR PROPOSALS

UK-3182.00-1-26

Procure Custom Indoor Batting Tunnel

ADDENDUM #1

08/26/2025

ATTENTION: This is not an order. Read all instructions, terms, and conditions carefully.

IMPORTANT: RFP AND ADDENDUM MUST BE RECEIVED BY: 09/09/2025 @ 3:00 P.M. LEXINGTON, KY TIME

Offeror should acknowledge receipt of this, and any addendum as stated in the Request for Proposal.

ITEM #1: ADDITIONS/REVISIONS TO THE ORIGINAL RFP DOCUMENTS

- Please see the enclosed additional information from the project team.

OFFICIAL APPROVAL
UNIVERSITY OF KENTUCKY

08/26/2025

Ken Scott

Ken Scott, Purchasing Officer

SIGNATURE

_____

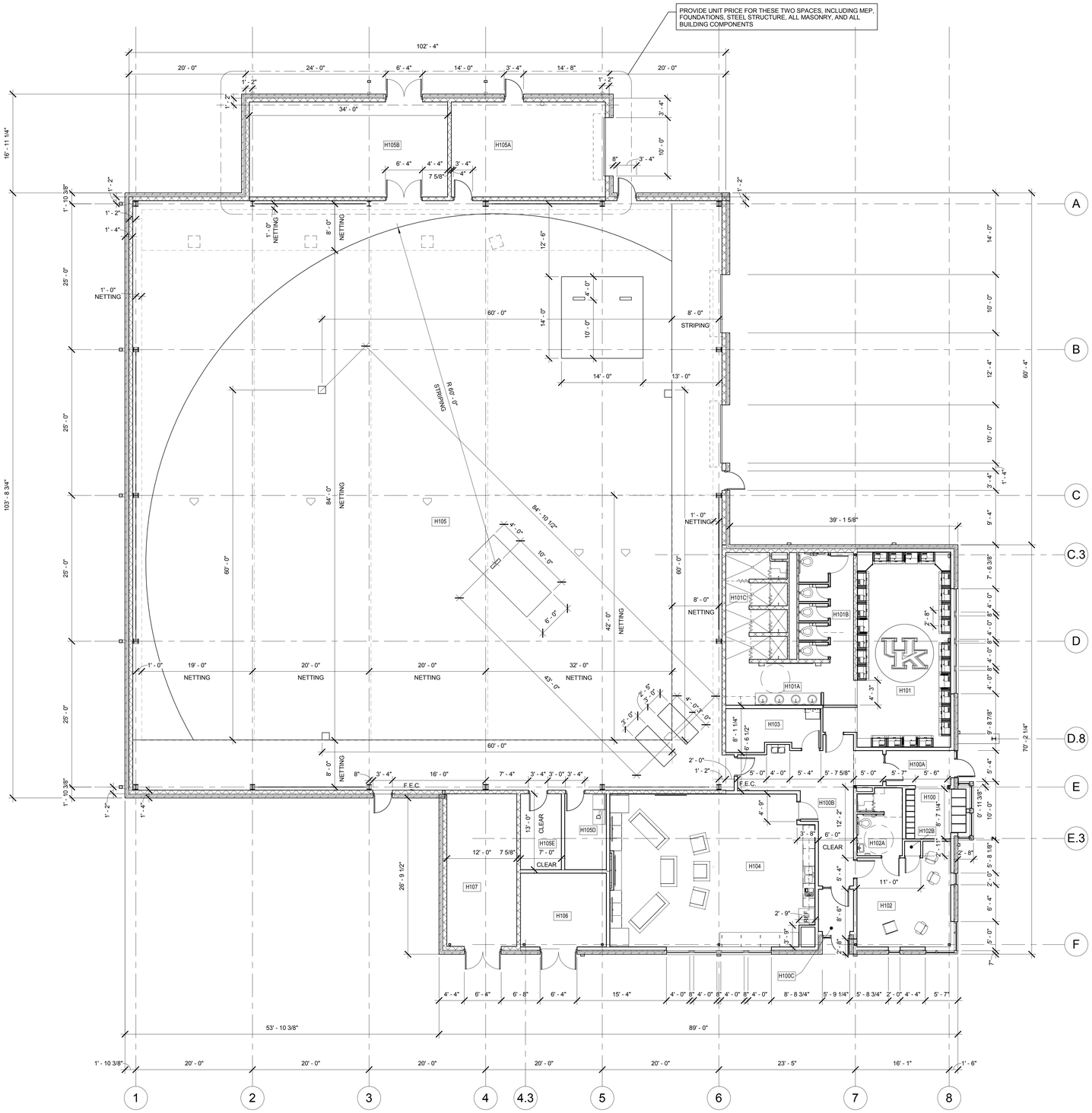
_____

Typed or Printed Name

University of Kentucky
Procurement Services
322 Peterson Service Building

Lexington, KY 40506-0005

HP ROOM NAME LIST		
NO.	NAME	NET SQ. FT.
H100	MUD ROOM	88 SF
H100A	VEST.	68 SF
H100B	CORRIDOR	295 SF
H100C	VEST.	43 SF
H101	LOCKERS	562 SF
H101A	LAVATORIES	164 SF
H101B	TOILETS	172 SF
H101C	SHOWER	177 SF
H102	OFFICE	258 SF
H102A	TOILET	85 SF
H102B	VIDEO CLOSET	7 SF
H103	JAN / MECH.	110 SF
H104	LOUNGE	890 SF
H105	HITTING PAVILION	10226 SF
H105A	FIELD STORAGE 2	431 SF
H105B	TEAM STORAGE 1	554 SF
H105D	TEAM STRG 2	93 SF
H105E	TELECOM	91 SF
H106	MECH. / ELEC.	186 SF
H107	FIRE PUMP	314 SF



PLAN NORTH

FIRST FLOOR - HITTING PAVILION - DIMENSIONS

SCALE: 1/8" = 1'-0"

CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

737 South Third Street, Louisville, Kentucky 40202-2100
502-585-4181 / 502-587-0488 Fax www.luckett-farley.com
Master Planning | Architecture | Engineering | Interior Design
Design Build | Building Commissioning | Special Inspections
[smart design]SM Luckett&Farley
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University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

COMM. NO.	2012.030.00
MADE BY	NWF
CHECKED	
DATE	5/30/12

HITTING PAVILION FIRST FLOOR - DIMENSIONS

DRAWING NO. **H-A2.1**
BID PKG.

5/30/2012 3:23:07 PM
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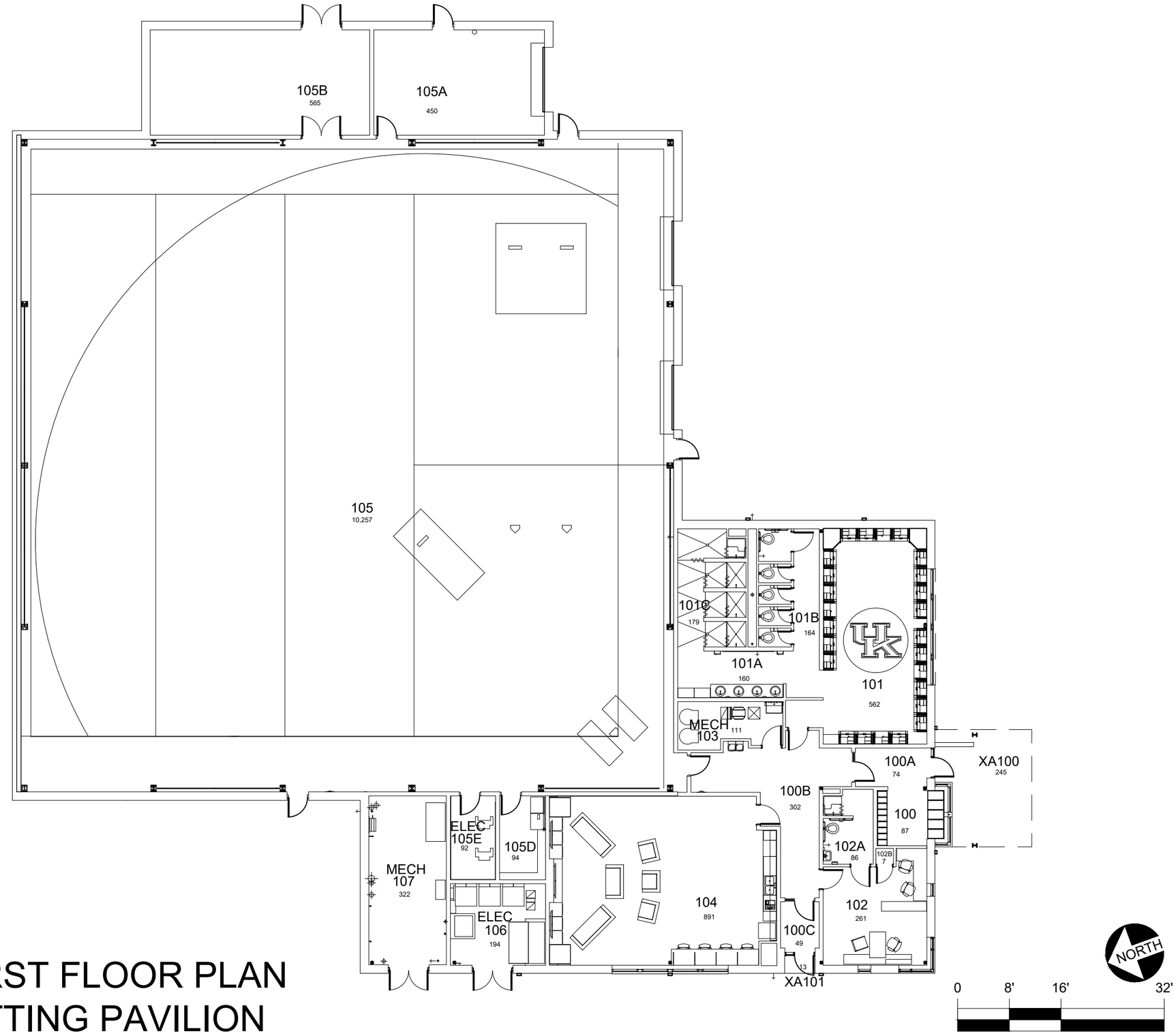


FIRST FLOOR PLAN
HITTING PAVILION

16,193 Gross Square Footage

University of Kentucky

BLDG #: 0447

HITTING PAVILLION

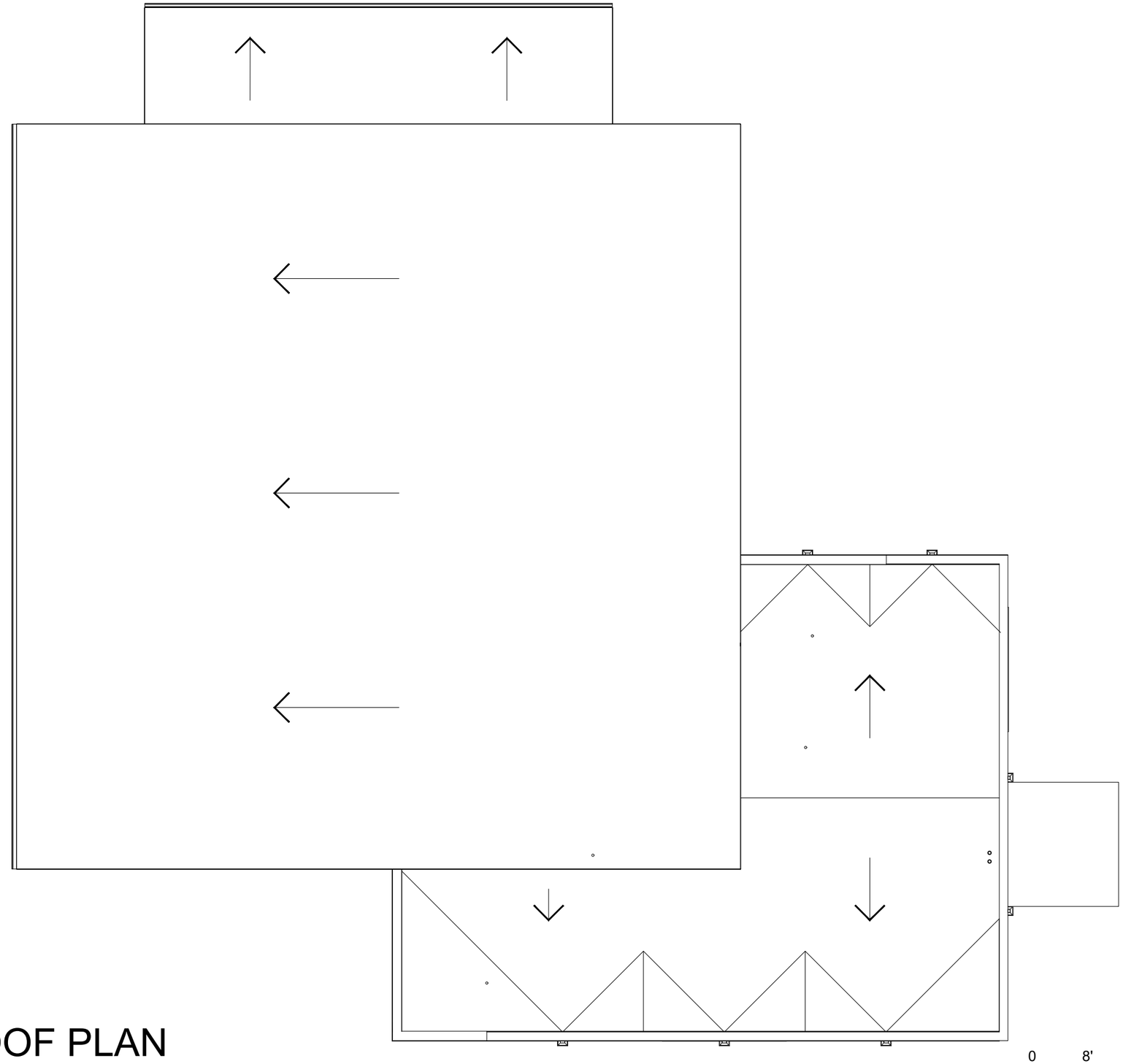
DRAWN BY: AAB

DATE: 06-21-19

REVISED BY: AAB

DATE: 06-21-19

SHEET 1 OF 2



ROOF PLAN
HITTING PAVILION

0 8' 16' 32'

NORTH

University of Kentucky

BLDG #: 0447

HITTING PAVILLION

DRAWN BY: AAB

DATE: 06-21-19

REVISED BY: AAB

DATE: 06-21-19

SHEET

2 OF 2

CONTRACTOR COORDINATION:

1. DISCREPANCIES - THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, AND EXISTING CONDITIONS (WHERE APPLICABLE) AT THE JOB SITE AS WELL AS THE PROVISIONS OF THE ENTIRE CONSTRUCTION DOCUMENTS AND BRING TO THE ARCHITECT'S ATTENTION ANY DISCREPANCY. IN THE EVENT OF A DISCREPANCY IN THE STRUCTURAL CONSTRUCTION DOCUMENTS, THE NOTE OR DETAIL UTILIZING THE STRICTER REQUIREMENT SHALL APPLY.
2. EXCAVATION, SHORING, AND BRACING - IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DESIGN AND PROVIDE ADEQUATE SHORING, BRACING, FORM WORK, ETC., AS REQUIRED FOR PROTECTION OF LIFE AND PROPERTY, TO SUPPORT ANY CONSTRUCTION LOADS, AND TO MAINTAIN ALL BUILDING COMPONENTS SAFELY IN PLACE PRIOR TO THEIR FINAL ASSEMBLY AND ANCHORAGE INTO THE COMPLETED STRUCTURE.
3. INSPECTIONS - ALL INSPECTION AND TESTING SHALL BE PERFORMED ACCORDING TO BUILDING CODE AND THE REQUIREMENTS OF THESE CONTRACT DOCUMENTS, WHICHEVER IS MORE STRINGENT.
4. COORDINATION - COORDINATE STRUCTURAL WORK WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, ELECTRICAL, PLUMBING AND ALL OTHER PERTINENT DRAWINGS FOR THE SIZE AND LOCATION OF PIPE, VENT, DUCT AND OTHER OPENINGS AND DETAILS NOT SHOWN ON THE STRUCTURAL DRAWINGS. ALL DIMENSIONS SHALL BE VERIFIED AND COORDINATED BY THE CONTRACTOR.
5. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION.

SHOP DRAWINGS:

1. SHOP DRAWINGS ARE AN AID FOR FIELD PLACEMENT AND ARE SUPERSEDED BY THE CONTRACT DOCUMENTS. ANY REVIEW OF SHOP DRAWINGS BY THE ENGINEER IS ONLY FOR GENERAL CONFORMANCE TO THE STRUCTURAL REQUIREMENTS AND IN NO WAY GUARANTEES THE ACCURACY OR COMPLETENESS OF INFORMATION THEREIN. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO INSURE ALL CONSTRUCTION IS IN FULL COMPLIANCE WITH THE CONTRACT DOCUMENTS.
2. SHOP DRAWINGS PREPARED USING A COPY OF ANY PART OF THE CONTRACT DOCUMENTS WILL BE REJECTED.
3. SUBMIT ONLY TWO (2) COPIES FOR REVIEW, ON SET WILL BE RETURNED WITH ANY COMMENTS. MAKE ALL REQUIRED COPIES FROM THE ONE REVIEWED SET.
4. SHOP DRAWINGS - SUBMIT SHOP DRAWINGS TO ENGINEER MINIMUM THREE WEEKS IN ADVANCE OF FABRICATION INDICATING ALL FIELD CONDITIONS. ALL SHEETS OF STEEL SHOP DRAWINGS MUST BE SIGNED AND SEALED BY PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION, FABRICATORS ENGINEER'S REVIEW SHALL INCLUDE VERIFICATIONS THAT HIS CONNECTION DESIGN HAS BEEN INCORPORATED IN THE SHOP DRAWING.

GEOTECHNICAL REPORT:

REFERENCE SHOULD BE MADE TO GEOTECHNICAL INVESTIGATION REPORT PREPARED BY:

AMEC ENVIRONMENTAL & INFASTRUCTURE, INC.
690 COMMONWEALTH CTR.
11003 BLUEGRASS PARKWAY
LOUISVILLE, KY 40299
DATED APRIL 20, 2012

GEOTECHNICAL ENGINEER SHALL BE RETAINED TO PROVIDE OBSERVATION AND TESTING SERVICES DURING THE GRADING AND FOUNDATION PHASE OF CONSTRUCTION PER GEOTECHNICAL REPORT RECOMMENDATIONS. INSPECTION AND TESTING REPORTS SHALL BE SUBMITTED TO THE ENGINEER.

DESIGN RECOMMENDATIONS ARE AS FOLLOWS:

ALLOWABLE BEARING PRESSURE: 2.0 KSF
PASSIVE PRESSURE: Kp = 3.0
ACTIVE PRESSURE: Ka = 0.33
AT REST: Ko = 0.5
FRICTION COEFFICIENT: 0.4
SOIL SUBGRADE MODULUS: 100 PCI
SITE CLASS: C

CONCRETE NOTES:

1. CONCRETE COMPRESSIVE STRENGTH - PROVIDE CONCRETE WITH THE FOLLOWING STRENGTHS AT THE LOCATIONS NOTED. MIX DESIGN, SLUMP, AIR ENTRAINMENT, AGGREGATE SIZE, ETC. SHALL BE IN CONFORMANCE WITH THE ACI 301, LATEST EDITION.

   LOCATION ... STRENGTH (PSI @ 28 DAYS)

   FOUNDATIONG & WALLS.. ....4000 PSI NORMAL WEIGHT
   SLABS ON METAL DECK (U.N.O) ....3500 PSI NORMAL WEIGHT
2. REINFORCING STEEL - ASTM A615 GRADE 60 (UNLESS WELDED)
   ALL BARS TO BE WELDED SHALL CONFORM TO ASTM A706
   WWR - ASTM A185
3. SPLICES - ALL SPLICES SHALL BE AS INDICATED ON SCHEDULE BELOW. CONTINUOUS REINFORCEMENT SPLICES SHALL BE CLASS "B" AND STAGGERED.
4. FABRICATE AND PLACE REINFORCEMENT IN ACCORDANCE WITH ACI PUBLICATION SP-66, ACI DETAILING MANUAL - LATEST EDITION.
5. PLACE CONCRETE IN COMPLIANCE WITH ACI 304. ALL CONCRETE SHALL BE MECHANICALLY VIBRATED.
6. CONCRETE COVER FOR REINFORCEMENT FOR NON-PRESTRESSED, CAST IN PLACE CONCRETE SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:

CONDITION	COVER
CAST AGAINST EARTH	3"
EXPOSED TO WEATHER	
#5 AND SMALLER	1 1/2"
#6 AND LARGER	2"
NOT EXPOSED TO WEATHER OR GROUND	
SLAB, WALL, JOISTS #11 BAR AND SMALLER	3/4"
BEAMS, COLUMNS	1 1/2"
SLAB-ON-GRADE	1 1/2" FROM TOP

7. REINFORCING SUPPORT - ALL REINFORCING (BARS AND WWR) SHALL BE ADEQUATELY CHAIRED/BOLSTERED. LIFTING OR HOOK IS NOT PERMITTED.
8. EMBEDS - ALL ITEMS TO BE CAST INTO CONCRETE SUCH AS REINFORCING DOWELS, BOLTS, ANCHORS, PIPES, SLEEVES, ETC., SHALL BE SECURELY AND ACCURATELY POSITIONED INTO THE FORMS PRIOR TO PLACING THE CONCRETE.
9. CONSTRUCTION JOINTS - THE CONTRACTOR SHALL OBTAIN THE ENGINEER'S APPROVAL FOR CONCRETE CONSTRUCTION JOINT LOCATIONS. REINFORCING STEEL DETAILING MAY CHANGE AND THE CONTRACTOR MAY BE RESPONSIBLE FOR ADDITIONAL EXPENSES AS A RESULT.
10. EDGE CHAMFER - ALL EXPOSED EDGES AND CORNERS OF THE CONCRETE SHALL HAVE A 3/4" CHAMFER AT 45° UNLESS NOTED OTHERWISE.

TENSION LAP SPLICES - CLASS B FOR TOP & BOTTOM BARS
(GRADE 60 UNCOATED BARS - NORMAL WEIGHT CONCRETE)

BAR SIZE	f'c=3000 psi		f'c=4000 psi	
	TOP	BOT.	TOP	BOT.
# 3	2'-4"	1'-9"	2'-0"	1'-6"
# 4	3'-1"	2'-4"	2'-8"	2'-1"
# 5	3'-10"	3'-0"	3'-4"	2'-7"
# 6	4'-8"	3'-7"	4'-0"	3'-1"
# 7	6'-9"	5'-2"	5'-10"	4'-6"
# 8	7'-9"	5'-11"	6'-8"	5'-2"
# 9	8'-8"	6'-8"	7'-6"	5'-10"
# 10	9'-10"	7'-6"	8'-6"	6'-6"
# 11	10'-11"	8'-4"	9'-5"	7'-3"

NOTES:
1. FOR CLASS 'A' SPLICE (PERMITTED ONLY WHEN NOT MORE THAN HALF THE BARS SPLICED & SPLICES STAGGERED BY THE DISTANCE OF SPLICE LENGTH), USE SAME AS 'ℓd' = TENSION DEVELOPMENT LENGTH TABLE.
2. VALUES AQUIRED UNDER ACI 318-05 PROVISIONS.
3. TOP BARS ARE DEFINED AS ANY BAR WITH MORE THAN 12" OF FRESH CONCRETE CAST BELOW THE SPLICED BARS.

NON-SHRINK GROUT NOTES:

1. NON-SHRINK GROUT SHALL BE HIGH STRENGTH, NON-FERROUS, NON-SHRINK GROUT MANUFACTURED BY MASTER BUILDERS (NSGROUT), OR APPROVED EQUIVALENT. SURFACE OF EXISTING CONCRETE SHALL BE FREE FROM DUST, DEBRIS OR WATER PRIOR TO PLACING GROUT. GROUT PRODUCT SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN RECOMMENDATIONS. GROUT MUST BE PLACED IMMEDIATELY AFTER COLUMN IS PLUMB, AND BEFORE ANY DECKING IS PLACED.
2. MINIMUM CONPRESSIVE STRENGTHS AT 28 DAYS SHALL BE (2) TIMES THE DESIGN COMPRESSIVE STRENGTH OF THE CONCRETE FOOTING OR MASONRY WALL, WHICHEVER IS LESS.

HIGH STRENGTH BOLTS (H.S.B.):

1. HIGH STRENGTH BOLTS SHALL CONFORM TO THE PROVISIONS OF THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS", LATEST ADDITION, AS APPROVED BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC) AND THE PROVISIONS OF THE APPLICABLE BUILDING CODE.
2. THE ASSEMBLY SURFACE, INCLUDING THOSE ADJACENT TO THE WASHER, SHALL BE FREE OF MILL SCALE, OIL, PAINT OR OTHER COATINGS.

ADHESIVE ANCHOR NOTES:

1. ACCEPTABLE PRODUCTS FOR USE IN (CRACKED) CONCRETE APPLICATIONS WHERE APPROVED BY REQUEST ARE AS FOLLOWS:

   ITW RED HEAD EPCON G5
   I.C.C. EVALUATION REPORT NO. 1137

   HILTI HY 150 MAX-SD
   I.C.C. EVALUATION REPORT NO. 3013

   SIMPSON SET-XP
   I.C.C. EVALUATION REPORT NO. 2508

   THE CONTRACTOR SHALL SUBMIT THE EPOXY ADHESIVE PRODUCT NAME TO BE USED ALONG WITH ITS ICC REPORT AS WELL AS A SKETCH OF THE CONDITION WHERE EPOXY IS PROPOSED, FOR REVIEW.
2. PROVIDE DRILLED HOLES OF DIAMETER AND DEPTH RECOMMENDED BY THE PRODUCT MANUFACTURER FOR THE DOWEL SIZE SPECIFIED OR OF DIAMETER AND DEPTH SPECIFIED IN THE CONTRACT DOCUMENTS, WHICHEVER IS GREATER. THOROUGHLY CLEAN HOLE OF CONCRETE DUST WITH BRUSH AND OIL-FREE COMPRESSED AIR. INJECT ADHESIVE PER THE MANUFACTURER'S RECOMMENDATIONS.
3. ENCOUNTERING EXISTING REINFORCING DURING DRILLING - CONTRACTOR SHALL NOT DRILL THROUGH REBAR IN EXISTING CONCRETE. CONTACT THE ENGINEER OF RECORD WHERE EXISTING REINFORCEMENT IS ENCOUNTERED DURING THE DRILLING PROCESS.

COLD FORM METAL FRAMING GENERAL NOTES:

1. THE CONTRACTOR SHALL ENGAGE A QUALIFIED PROFESSIONAL ENGINEER TO DESIGN ALL COLD ALL COLD FORMED METAL FRAMING MAMBERS AND ASSOCIATED CONNECTIONS IN ACCORDANCE WITH THE NORTH AMERICAN COLD FORMED STEEL SPECIFICATION 2001 EDITION.
2. COLD FROMED METAL FRAMING SHALL BE DESGINED TO RESIST THE LOADS INDICATED IN THE DESIGN CRITERIA ON SHEET S0.0.
3. DESIGN CALCULATIONS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW WITH THE SHOP DRAWING PACKAGE.
4. IN ALL DOUBLE JAMB STUDS AND DOUBLE HEADERS NOT ACCESSIBLE TO INSULATION CONTRACTORS, INSULATION EQUAL TO THAT SPECIFIED ELSEWHERE WITHIN THE WALL SHALL BE PROVIDED. COORDINATE WITH ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR INSULATION REQUIREMENTS.
5. SPACING OF STUDS SHALL BE COORDINATED WITH THE PRE-ENGINEERED LIGHT GAGE TRUSS LOCATIONS. TRUSSES SHALL BEAR ON STUDS UNLESS ADEQUATE MEANS ARE DESIGNED AND PROVIDED FOR SUPPORTING THE TRUSS BETWEEN THE STUDS.

STEEL DECK NOTES:

1. ROOF AND FLOOR DECKS SHALL BE PROVIDED AND FASTENED AS NOTED ON THE DRAWINGS.
2. DECK SHOP DRAWINGS SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL PRIOR TO FABRICATION AND SHALL INDICATE STUD LAYOUT, IF APPLICABLE.
3. THE AMERICAN IRON AND STEEL INSTITUTE "SPECIFICATIONS FOR THE DESIGN OF LIGHT GAUGE STEEL STRUCTURAL MEMBERS" SHALL GOVERN THE DESIGN OF ALL DECK UNITS, STEEL DECK AND ALL OF ITS CLOSURES AND FLASHINGS SHALL CONFORM TO ASTM A446, GRADE B, Fy = 38,000 PSI MINIMUM.
4. DECK UNITS SHALL BE CONTINUOUS OVER THREE OR MORE SPANS. NOTIFY THE ENGINEER OF RECORD OF THE LOCATIONS/CONDITIONS WHERE THIS IS NOT FEASIBLE. SHORING MAY BE REQUIRED AT NON-CONTINUOUS SPANS. DECK SHOP DRAWINGS SHALL INDICATE WHERE SHORING WILL BE REQUIRED. DECK SHALL BEAR A 2" MINIMUM AT ALL SUPPORTS. ALL DECKS FOR INTERIOR FLOORS SHALL HAVE VENT TABS FOR CONCRETE VENTILATION.
5. ALL WELDING OF STEEL DECK SHALL BE PERFORMED BY CERTIFIED LIGHT GAGE WELDERS IN ACCORDANCE WITH AWS "SPECIFICATIONS FOR WELDING SHEET STEEL IN STRUCTURES", AWS D1.3, LATEST EDITION.
6. PROVIDE FLASHING AND CLOSURE PLATES AT ENDS OF ALL UNITS, AROUND COLUMNS, AND AT ALL PERIMETER LOCATIONS REQUIRING CONCRETE.
7. PROVIDE CONTINUOUS SUPPORT FOR DECK EDGES WHETHER SHOWN OR NOT, UNLESS OTHERWISE NOTED.
8. PUDDLE WELDS SHALL HAVE AN EFFECTIVE MINIMUM DIAMETER OF 5/8 INCH.
9. CONTRACTOR SHALL VERIFY AND COORDINATE ALL FLOOR OPENING LOCATIONS AND DIMENSIONS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING WORK.
10. DECK ENDS SHALL BEAR A MINIMUM OF 2" AT SUPPORTS. DECKING SHALL HAVE BUTT JOINTS.

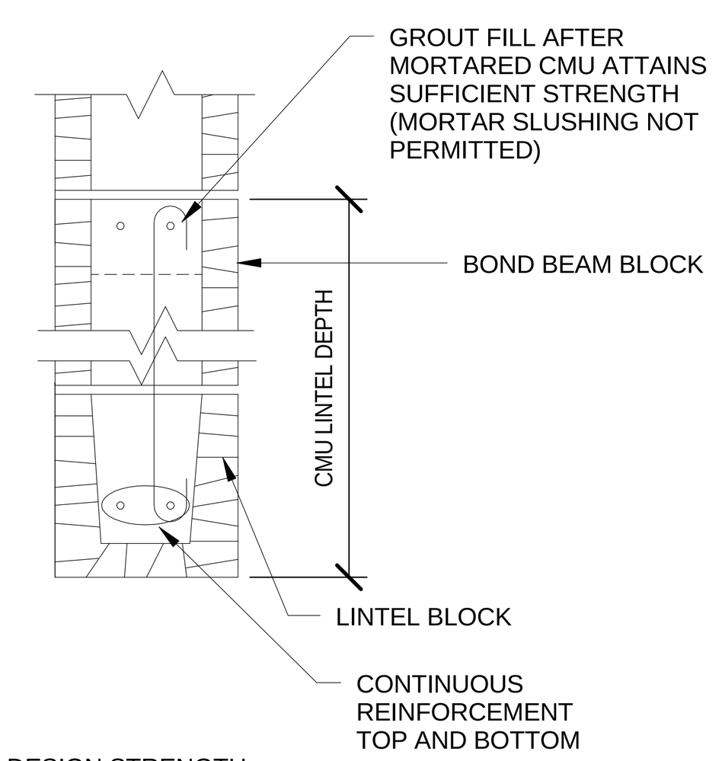
CONCRETE MASONRY GENERAL NOTES:

1. PROVIDE TYPICAL VERTICAL AND HORIZONTAL WALL REINFORCEMENT PER ELEVATIONS, PLANS, SECTIONS AND TYPICAL WALL REINFORCING DETAILS AT OPENINGS.
2. CONSTRUCT MASONRY IN RUNNING BOND ONLY, UNLESS NOTED OTHERWISE.
3. DO NOT PROVIDE VERTICAL CONTROL JOINTS WITHIN STRUCTURAL CMU WALLS (EXTERIOR, BEARING, OR SHEAR WALLS) , UNLESS SPECIFICALLY NOTED ON ELEVATIONS.
4. THE MASONRY WORK HAS BEEN DESIGNED AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530/ASCE 5/ TMS 402.)
5. CONCRETE BLOCK SHALL BE HOLLOW LOAD-BEARING CONCRETE MASONRY UNITS NORMAL WEIGHT, CONFORMING TO ASTM C90, GRADE N, WITH MINIMUM AVERAGE NET AREA COMPRESSIVE STRENGTH OF 1900 PSI BUT NOT LTHAN 1800 PSI UNLESS NOTED OTHERWISE.
6. MORTAR: TYPE S. CONFORM TO ASTM C270 WITH THE FOLLOWING VOLUMETRIC PROPORTIONS: 4 1/2 PARTS SAND, 1/2 PART HYDRATED LIME, 1 PART PORTLAND CEMENT.
7. THE REINFORCEMENT SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING SPECIFICATIONS:

   REBAR ... ASTM A615 GRADE 60
   GALVANIZED CARBON STEEL WIRE ... ASTM A82
8. GROUT REINFORCED MASONRY WITH LOW-LIFT METHOD ONLY (MAXIMUM LIFT HEIGHT 4'-0"). GROUT SHALL CONFORM TO ASTM C476 WITH THE FOLLOWING VOLUMETRIC PROPORTIONS FOR COURSE GROUT: 3 PARTS SAND, 2 PARTS PEA GRAVEL, 1 PART PORTLAND CEMENT & CLEAN, POTABLE WATER AS REQUIRED TO PRODUCE A SLUMP OF 8" - 11". MORTAR SHALL NOT BE USED AS GROUT.
9. MISCELLANEOUS:
   A. ALL REINFORCED CELLS SHALL BE FULLY-GROUTED, SOLID.
   B. PROVIDE A MINIMUM OF 1/2" CLEAR BETWEEN MAIN REINFORCING AND MASONRY UNITS.
   C. SET WELD PLATES IN BOND BEAMS AFTER THE GROUT IS PLACED, BUT WHILE IT IS STILL PLASTIC.
   D. SOLID UNITS TO BE LAID WITH FULL HEAD AND BED JOINTS.
   E. PROVIDE APPROPRIATE MASONRY ANCHORS AT 16" OC MAX TO TIE MASONRY TO ABUTTING VERTICAL STEEL AND CONCRETE SURFACES.
   F. ALL SPLICES FOR VERTICAL WALL REINFORCING SHALL BE CONTINUOUS AND ARE TO BE LAPPED ACCORDING TO THE SCHEDULE WITHIN THE 'CMU TYPICAL DETAILS' SHEET(S).
10. THE PRACTICE OF 'WET-STICKING' BARS DURING THE GROUT OPERATION IS NOT PERMITTED.
11. ALL GROUTED CELLS SHALL BE PROPERLY, MECHANICALLY VIBRATED PER ACI 530.

CONCRETE MASONRY LINTEL SCHEDULE:

SPAN	NO. COURSES	REINF, UNO ON ELEV.
L1	1 (8" LINTEL)	2) #5
L2	2 (16" LINTEL)	2) #5
L3	3 (24" LINTEL)	2) #5



1. LINTEL MUST BE SHORED UNTIL MORTAR AND GROUT ATTAIN DESIGN STRENGTH.
2. SPECIAL INSPECTOR MUST VERIFY PROPER REINFORCEMENT PLACEMENT PRIOR TO GROUTING, AND VERIFY PROPER GROUT PLACEMENT.

LOOSE BRICK LINTEL NOTES:

1. ALL LOOSE BRICK VENEER SHALL BE HOT DIP GALVANIZED.
2. PROVIDE 6" BEARING ON EACH END OF STEEL LINTEL MINIMUM.
3. LOOSE BRICK VENEER LINTEL MINIMUM SIZE SHALL BE AS FOLLOWS:
   UP TO 6'-6"... L3x3x3/8
   UP TO 8'-0"... L5x3 1/2x3/8"
   OVER 8'-0" @ CMU.................................. PER 14/S-S5.3

STEEL BAR JOIST NOTES:

1. SPECIFICATIONS:
   A. FABRICATION AND ERECTION TO BE PER SJI STANDARD SPECIFICATION REQUIREMENTS.
   B. MANUFACTURER TO BE A MEMBER OF SJI.
2. BRIDGING:
   A. SPACING AS SHOWN ON THE CONTRACT DRAWINGS, BUT NOT LESS THAN REQUIRED BY SJI. UNLESS NOTED OTHERWISE PROVIDE THE FOLLOWING BRIDGING:
      1. K-SERIES:
         a. USE HORIZONTAL BRIDGING FOR K-SERIES.
         b. HORIZONTAL BRIDGING MAY BE WELDED TO THE JOISTS. DIAGONAL BRIDGING SHALL BE BOLTED TO JOISTS.
   B. DIAGONAL BRIDGING TO BE BOLTED TO THE JOISTS AND AT THEIR POINT OF INTERSECTION.
   C. END BAYS OF DIAGONAL BRIDGING TO BE ANCHORED WITH HORIZONTAL BRIDGING, UNLESS SHOWN OTHERWISE.
   D. HORIZONTAL BRIDGING IN NO MORE THAN 2 CONSECUTIVE BAYS MAY BE USED TO PROVIDE PASSAGE FOR DUCT WORK.
   E. ANCHOR BRIDGING TO INTERSECTING STRUCTURAL STEEL OR MASONRY WALLS.
3. BEARING:
   A. WELD ALL JOISTS TO SUPPORTING STEEL WITH 2" OF 1/8" FILLET WELD FOR OPEN WEB STEEL JOISTS AND 2" OF 1/4" FILLET WELD FOR LONG SPAN JOISTS, EACH SIDE OF BEARING. JOISTS TO BE FIELD BOLTED AT COLUMN LINES. EXTEND BOTTOM CHORDS OF THE SAME JOISTS AND BOLT THE TO THE BEAM OR COLUMN.
   B. EXTEND ALL JOISTS 1" MINIMUM PAST CENTERLINE OF SUPPORTING MEMBER, WHERE POSSIBLE. BEARINGS TO BE PER DRAWINGS, OR WHERE SPECIAL INSTRUCTION IS NOT GIVEN, ACCORDING TO THE STANDARD SPECIFICATIONS OF SJI.
4. MISCELLANEOUS:
   A. ADJACENT JOISTS OF THE SAME DEPTH ARE TO HAVE WEB MEMBERS IN LINE TO PERMIT PASSAGE OF MECHANICAL DUCTS.
   B. SEE DRAWINGS FOR SPECIAL BEARING SHOES, EXTENDED ENDS, ETC.
   C. COMPLETE STEEL ROOF JOIST LAYOUTS, SHOP DRAWINGS, AND CALCULATIONS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION. (SEE SHOP DRAWING NOTES).
   D. THE JOIST MANUFACTURER IS RESPONSIBLE FOR SUPPLYING JOIST INFORMATION SPECIFICATIONS INCLUDING PERMANENT BRACING SPECS.
   E. ANY DISCREPANCIES BETWEEN JOIST LAYOUTS AND CONTRACT DOCUMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR EVALUATION PRIOR TO FABRICATION.

REINFORCING STEEL (FOR CONCRETE AND MASONRY):

1. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 19 OF THE CODE, ASTM A615, GRADE 60 UNO. DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A-305.
2. BARS SHALL BE CLEAN OF RUST, GREASE, OR OTHER MATERIALS LIKELY TO IMPAIR BOND. ALL REINFORCING BAR BENDS SHALL BE MADE COLD.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. PROVIDE LAPS AS PER ACI 318 SECTION 12.19, 9" MINIMUM.
4. REINFORCING BAR SPLICES SHALL BE MADE AS INDICATED ON THE DRAWINGS. MINIMUM SPLICE LENGTH FOR REINFORCING STEEL BARS IN MASONRY SHALL BE 48 BAR DIAMETERS, 24" MINIMUM. MINIMUM SPLICE LENGTH FOR REINFORCING STEEL BARS IN CONCRETE SHALL BE AS REQUIRED FOR CLASS B SPLICES PER ACI 318 SECTION 12.15.1 UNO. LAP ALL HORIZONTAL BARS AT CORNERS AND INTERSECTIONS.
5. ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL IN-PLACE INSPECTION IS MADE.
6. WHERE WELDING OF REINFORCING IS APPROVED BY THE STRUCTURAL ENGINEER, IT SHALL BE DONE BY AWS CERTIFIED WELDERS USING APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF STRUCTURAL WELDING CODE-REINFORCING STEEL AWSD1.4, LATEST REVISION. REINFORCING BARS TO BE WELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-706.
7. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE "ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," LATEST EDITION.

STRUCTURAL STEEL NOTES:

1. MATERIAL - MATERIAL SHALL BE IN ACCORDANCE WITH THE FOLLOWING ASTM DESIGNATIONS UNLESS NOTED OTHERWISE:

   WIDE FLANGE SHAPES...................................... A992 GRADE 50
   CHANNELS, PLATES AND BARS.......................... A36
   STEEL TUBES.. A500 GRADE B
   ANGLES A36
   ANCHOR RODS .... F1554 GRADE 36
   WELDING ELECTRODES .... SERIES E70
   PIPE.. A53 GRADE B
2. WELDING - PERFORM WELDING IN ACCORDANCE WITH AWS D1.1 2008. WELDS SHALL BE MADE ONLY BY OPERATORS CERTIFIED BY AWS IN PERFORMING THE TYPE OF WORK INDICATED.
3. WELDING CONSUMABLES - WELD CONSUMABLES SHALL HAVE YIELD AND TENSILE STRENGTH EQUAL TO OR SLIGHTLY GREATER THAN THE EXPECTED YIELD AND TENSILE STRENGTH OF STEEL MEMBER BEING WELDED.
4. BOLTING - BOLTED CONNECTIONS SHALL CONFORM TO THE RCSC- "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325 OR A490 BOLTS". ALL BOLTS SHALL BE 3/4" DIA ASTM A325-N UNLESS NOTED OTHERWISE. PROVIDE MINIMUM EDGE DISTANCE AND BOLT SPACING PER AISC 360-05. THE ASSEMBLY SURFACE, INCLUDING THOSE ADJACENT TO THE WASHER, SHALL BE FREE OF MILL SCALE, OIL, PAINT, OR OTHER SURFACES.
5. EXPOSED STEEL - STEEL EXPOSED TO THE ELEMENTS SHALL BE GALVANIZED. GALVANIZING SHALL CONFORM TO ASTM A-123 FOR ROLLED, PRESSED OR FORGED SHAPES AND PLATES, ASTM A-153 FOR HARDWARE ITEMS, AND ASTM A-386 FOR ASSEMBLED STEEL PRODUCTS. RELIEF ANGLES ARE CONSIDERED EXPOSED. BOLTS SHALL BE GALVANIZED.
6. PRIME PAINTING - AFTER MATERIAL HAS BEEN PROPERLY CLEANED AND TREATED, APPLY SHOP PRIME COAT TO ALL SURFACES, EXCEPT THOSE INTENDED FOR EMBEDMENT INTO CONCRETE OR TO RECEIVE FIELD WELDING, SLIP CRITICAL BOLTS. PROVIDE "TOUCH-UP" AT SITE. MINIMUM DRY FILM THICKNESS SHALL BE 1.5 MIL.
7. FABRICATION - ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, LATEST EDITION.
8. METAL DECKING - METAL DECKING SHALL CONFORM TO ASTM A446, GRADE C, OR ASTM A611 GRADE D, WITH A 40000 PSI MINIMUM YIELD. THE DECK SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A525 GLASS G-60 COATING.

ABBREVIATIONS:

AFF	= ABOVE FINISHED FLOOR	MECH	= MECHANICAL
A/C	= AIR CONDITIONING	MFR	= MANUFACTURER
B/	= BOTTOM OF	MIN	= MINIMUM
BLW	= BELOW		
BM	= BEAM	MISC	= MISCELLANEOUS
BOT	= BOTTOM	NIC	= NOT IN CONTRACT
BP	= BASE PLATE	NTS	= NOT TO SCALE
CIP	= CAST IN PLACE		
CMU	= CONCRETE MASONRY UNIT	OC	= ON CENTER
COL	= COLUMN	OD	= OUTSIDE DIAMETER
CONC	= CONCRETE	OF	= OUTSIDE FACE
CJ	= CONSTRUCTION OR CONTROL JOINT	P	= PILE
		PL	= PLATE
CY	= CUBIC YARD	PC	= PILE CAP
DIA	= DIAMETER	PR	= PIER
DIM	= DIMENSION	OPNG	= OPENING
EL	= ELEVATION	OPP	= OPPOSITE
EJ	= EXPANSION JOINT	PCF	= POUNDS/CUBIC FOOT
EQ	= EQUAL	PLF	= POUNDS/LINEAR FOOT
EX	= EXISTING	PSF	= POUNDS/SQUARE FOOT
EXT	= EXTERIOR	PSI	= POUNDS/SQUARE INCH
EW	= EACH WAY	PT	= POST TENSION
EF	= EACH FACE	PVC	= POLYVINYL CHLORIDE
FD	= FLOOR DRAIN	R	= RADIUS
FF	= FINISHED FLOOR	RCP	= REINFORCED CONCRETE PIPE
FLR	= FLOOR	RD	= ROOF DRAIN
FND	= FOUNDATION	REINF	= "REINFORCED, REINFORCING"
FTG	= FOOTING	REF	= REFERENCE
GA	= GAUGE	RE	= REVISION
GALV	= GALVANIZED	REQ	= REQUIRED
GB	= GRADE BEAM	SCHD	= SCHEDULE
HDG	= HOT DIPPED GALVANIZED		
HP	= HIGH POINT	SF	= SQUARE FOOT
HSA	= HEADED STUD ANCHOR	SIM	= SIMILAR
HSS	= HOLLOW STRUCTURAL STEEL	SPA	= "SPACE, SPACING"
HORZ	= HORIZONTAL	SQ	= SQUARE
HT	= HEIGHT	SS	= STRUCTURAL STEEL
HVAC	= "HEATING, VENTILATION, AND A/C"	SSTL	= STAINLESS STEEL
		STD	= STANDARD
ID	= INSIDE DIAMETER	STL	= STEEL
IF	= INSIDE FACE	SYM	= SYMMETRICAL
INT	= INTERIOR	T/	= TOP OF
JST	= JOIST	T&B	= TOP AND BOTTOM
JT	= JOINT	TEMP	= TEMPORARY
L	= LENGTH	TYP	= TYPICAL
LF	= LINEAR FOOT	VERT	= VERTICAL
L&F	= LUCKETT & FARLEY	VIF	= VERIFY IN FIELD
LLBB	= LONG LEG BACK TO BACK	UNO	= UNLESS NOTED OTHERWISE
LLH	= LONG LEG HORIZONTAL	W	= WIDTH
LLV	= LONG LEG VERTICAL	W/	= WITH
LP	= LOW POINT	W/O	= WITHOUT
MAX	= MAXIMUM	WP	= WORK POINT
		WWR	= WELDED WIRE REINFORCEMENT

DESIGN CRITERIA		
LOAD TYPE	COMPONENT	LOAD VALUE
BUILDING CODE:	2006 INTERNATIONAL BUILDING CODE	
DEAD LOADS:		
	CONCOURSE FLOOR	
	4 1/2" THICK (AVG.) N.W. SLAB OVER 1" DECK (w/ 1/4" OVERLOAD)	65 PSF
	FRAMING SELF WEIGHT	5 PSF
	COLLATERAL	10 PSF
		TOTAL = 80 PSF
	PRESS FLOOR	
	2 1/2" THICK N.W. SLAB OVER 2" METAL DECK (w/ 1/4" OVERLOAD)	48 PSF
	FRAMING SELF WEIGHT	4 PSF
	COLLATERAL	10 PSF
		TOTAL = 62 PSF
	HITTING PAVILION & STADIUM - ROOF	
	ROOF DECKING	3 PSF
	ROOF FRAMING SELF-WEIGHT	4 PSF
	INSULATION	3 PSF
	COLLATERAL	10 PSF
		TOTAL = 20 PSF
	DUGOUT	
	6" THICK NORMAL WEIGHT SLAB	75 PSF
ICE DEAD LOAD:		
	SUPPORT POLES FOR STADIUM BACKSTOP NETTING HAS BEEN DESIGNED FOR ICE WEIGHT BASED ON A 25 YEAR MEAN OCCURANCE	
	ICE LOAD ON NET	7 PSF
LIVE LOADS:		
	CONCOURSE FLOOR	140 PSF
	PRESS FLOOR	100 PSF
	DUGOUT ROOF	100 PSF
	ROOF	20 PSF
SNOW LOADS:		
	GROUND SNOW LOAD	15 PSF
	EXPOSURE FACTOR (Ce)	0.9
	THERMAL FACTOR (Ct)	1.0
	IMPORTANCE FACTOR (Is)	1.0
	FLAT ROOF SNOW LOAD	9.45 PSF
WIND LOADS:		
	BASIC WIND SPEED (3-SECOND GUST)	90 MPH
	EXPOSURE CATEGORY	C
	IMPORTANCE FACTOR (Iw)	1.0
	INTERNAL PRESSURE COEFFICIENT	+/- 0.18
	COMPONENT & CLADDING PRESSURE	SEE TABLE BELOW
SEISMIC LOADS:		
	SPECTRAL RESPONSE ACCELERATIONS	
	Ss	0.228
	S1	0.088
	SITE CLASS	C
	SPECTRAL RESPONSE COEFFICIENTS	
	Sds	0.183
	Sd1	0.099
	IMPORTANCE FACTOR (Ie)	1.0
	SEISMIC DESIGN CATEGORY	B
	SEISMIC BRACING OF NON-STRUCTURAL (ARCHITECTURAL, MECHANICAL, ELECTRICAL, & PLUMBING) COMPONENTS IS NOT REQUIRED IN ACCORDANCE WITH CHAPTER 13 OF ASCE 7-05	
	BASIC SEISMIC-FORCE-RESISTING SYSTEM:	STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
	RESPONSE MODIFICATIONS COEFFICIENTS (R)	3.0
	SEISMIC RESPONSE COEFFICIENT (Cs)	0.061
	DESIGN BASE SHEAR	
	- HITTING PAVILION	V= 25 k
	- STADIUM	V = 68 k
	ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE

HITTING PAVILION COMPONENTS & CLADDING WIND PRESSURES POSITIVE AND NEGATIVE VALUES INDICATE NORMAL SURFACE PRESSURES TOWARDS & AWAY RESPECTIVELY			
ZONE	EFFECTIVE WIND AREA (SQ FT)	POSITIVE PRESSURE (PSF)	NEGATIVE PRESSURE (PSF)
ROOF INTERIOR ZONE (>10' FROM EDGE OR RIDGE)	10	13.1	-20.9
	20	11.6	-20.1
	50	10.4	-19.5
	100	10	-18.9
ROOF END ZONE (<10' FROM EDGE OR RIDGE)	10	13.1	-36.3
	20	11.6	-32.5
	50	10.4	-29.6
	100	10	-26.7
ROOF CORNER ZONE (WITHIN 10' OF ROOF CORNER)	10	13.1	-53.7
	20	11.6	-49.1
	50	10.4	-45.6
	100	10	-42.1
ROOF OVERHANG (>10' FROM CORNER OF ROOF)	10	0	-46
	20	0	-46
	50	0	-46
	100	0	-46
ROOF OVERHANG (WITHIN 10' OF CORNER OF ROOF)	10	0	-75
	20	0	-65.8
	50	0	-58.8
	100	0	-51.8
WALL INTERIOR ZONE (>10' FROM CORNER OF BUILDING)	10	22.8	-24.7
	20	21.4	-23.4
	50	20.4	-22.4
	100	19.4	-21.3
WALL END ZONE (WITHIN 10' OF CORNER OF BUILDING)	10	22.8	-30.5
	20	21.4	-27.8
	50	20.4	-25.8
	100	19.4	-23.7

COMPONENTS DESIGNED BY OTHER DESIGN PROFESSIONALS:

THE CONTRACTOR SHALL PROVIDE THE FOLLOWING LIST OF BUILDING COMPONENTS AND/OR EQUIPMENT TO BE DESIGNED AND DETAILED BY A PROFESSIONAL STRUTURAL ENGINEER LICENSED IN THE COMMONWEALTH OF KENTUCKY, AND COMPLY WITH THE 2007 KBC AND ASCE 7-05 CH. 13. STAMPED AND SIGNED SHOP DRAWINGS, ANCHORAGE DETAILS TO THE SUPPORT STRUCTURE, CALCULATIONS, PROPRIETARY HARDWARE INFORMATION, ETC. SHALL BE SUBMITTED, REVIEWED, AND APPROVED BY THE DESIGN TEAM PRIOR TO FABRICATION OR PROCUREMENT, AS PER THE PROJECT SPECIFICATIONS. PROJECT AND SITE-SPECIFIC DESIGN PARAMETERS PROVIDED IN THE CONSTRUCTION DOCUMENTS ARE TO BE UTILIZED IN THE PREPARATION OF THE SUBMITTAL DOCUMENT(S):

- COLD FORMED METAL FRAMED WALLS
- INTERIOR METAL STUD PARTITIONS, CEILING STUDS (SSMA), AND CORRESPONDING DETAILS DETAILS.
  NOTE THAT THE USE OF SHOT PINS TO SUSPEND/HANG METAL STUDS FROM STRUCTURAL STEEL IS **NOT** PERMITTED.
- MECHANICAL AND ELECTRICAL EQUIPMENT:
- -SPRINKLER SYSTEMS SUPPORTED AND/OR BRACED AS REQUIRED PER NFPA 13
- -ELEVATOR & HOISTWAY SYSTEMS AS PER ASME A17.1
- ELEVATOR GUIDERAILS
- STEEL STAIRS AND HANDRAILS.
- NETTING HARDWARE AND ASSEMBLIES. PROVIDE L&F WITH REACTIONS/FORCES AT THE ATTACHMENTS TO STRUCTURE TO CONFIRM/VERIFY ALIGNMENT WITH THE FORCES USED IN THE ORIGINAL DESIGN.
- ALUMINUM BLEACHERS AND SEATING, INCLUDING CONNECTIONS TO CONCRETE PIERS/WALLS PROVIDED.

NOTE THAT EXTERIOR WALL SYSTEM SHALL BE CAPABLE OF ACCOMMODATING A MAX. INELASTIC STORY DRIFT (FLOOR-TO-FLOOR) OF 1/4".

CONSTRUCTION DOCUMENTS

0 1/2" 1"
SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

STATE OF KENTUCKY
KYLE S. BEASLEY
23151
LICENSED PROFESSIONAL ENGINEER

737 South Third Street, Louisville, Kentucky 40202-2100
502-585-4181 502-587-0488 Fax www.luckett-farley.com
Master Planning | Architecture | Engineering | Interior Design
Design Build | Building Commissioning | Special Inspections
1853
[smart design] SM
Luckett & Farley

University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

COMM. NO. 2012.030.00
MADE BY HRG
CHECKED
DATE 5/30/12

GENERAL NOTES

DRAWING NO. **S0.0**
BID PKG.

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SPECIAL INSPECTION SCOPE

MATERIAL	APPLICABLE TO THIS PROJECT	COMPONENT	REQUIRED	TEST OR SPECIAL INSPECTION	TYPE	PERFORMED BY	CODE REFERENCE AND NOTES
A. SOILS	X	1. GENERAL	X	**a.** Verify that: • site has been prepared properly prior to placement of controlled fill and/or excavations for foundations, • foundation excavations are extended to proper depth and have reached proper material, and • materials below footings are adequate to achieve the design bearing capacity.	P	GE	By geotechnical engineer or their qualified representative.
	X	2. COMPACTED FILLS		**a.** Perform qualification testing of fill materials.	T	Lab	Under the supervision of the geotechnical engineer.
			X	**b.** Verify use of proper materials and inspect lift thicknesses, placement, and compaction during placement of fill.	C	GE	By geotechnical engineer or their qualified representative.
			X	**c.** Test compaction of fill.	T	Lab	Under the supervision of the geotechnical engineer.
	X	5. RETAINING WALLS:		**a.** Placement of soil reinforcement, drainage devices, and backfill.	C	GE*	Placement, compaction and inspection of backfill per Section 1704.7.1 for fills supporting foundations (see A.2 above). Not required if fill is less than 12" deep.
				b. Segmental retaining walls; inspect placement of units, dowels, connectors, etc.	C	GE	By geotechnical engineer or their qualified representative.
				c. Concrete retaining walls.	Provide tests and inspections per CONCRETE section below.		
			X	**d.** Masonry retaining walls.	Provide tests and inspections per MASONRY section below.		
B. CONCRETE (1704.4, see Exemptions')	X	1. CAST-IN-PLACE (CIP) CONCRETE	X	**a.** Verify use of required design mix.	P	PI	
			X	**b.** Inspect reinforcing steel material.	P	SI	Table 1704.4.1, 1704.4.1
			X	**c.** Perform slump, temperature, and air content tests.	T	Lab	Table 1704.1 item 5. ASTM C172, ASTM C31.
			X	**d.** Test concrete (compression).	T	Lab	1905.6 ASTM C39.
			X	**f.** Inspect placement of formwork, reinforcing steel, embedded items and concrete. Inspect curing and form removal. Review hot and cold weather protection procedures.	P	SI	1906, 1907
			X	**g.** Welding of reinforcing steel.	Provide special inspection per STEEL, below.		
				h. Verify in-situ concrete strength prior to removal of shores and forms from beams and structural slabs.	N/A	PI	1906.2.2. Project inspector to verify concrete strength test reports prior to removal of shores or formwork.
		2. PRESTRESSED CONCRETE		**a.** Verify prestressing tendon material	P	SI	1704.4.1
				b. Inspect placement of prestressing tendons.	P	SI	Table 1704.4.1
				c. Verify in-situ concrete strength prior to stressing of tendons.	N/A	SI	Special inspector to verify concrete strength test reports prior to the stressing of post tensioned tendons
				d. Inspect application of prestressing forces and grouting of bonded prestressing tendons.	C	SI	Table 1704.4 item 8
		3. PRECAST CONCRETE		**a.** Inspect erection of precast concrete members.	P	SI	Table 1704.4 item 9
		4. SHOTCRETE		**a.** Inspect shotcrete placement.	C	SI	Table 1704.4 item 6. See ACI 506.
				b. Preconstruction test	C	SI	1914.5 ASTM C42, ASTM C1140.
				c. Strength test	T	Lab	1914.10 ASTM C42, ASTM C1140.
	X	5. POST-INSTALLED ANCHORS IN CONCRETE	X	**a.** Inspect installation of post-installed anchors	C	SI	As required by hardware's ICC Report (ESR) indicated on the construction drawings.
C. MASONRY (1704.5, see Exeption')	X	1. STRUCTURAL MASONRY		**a.** Inspect reinforcing steel material.	P	SI	Table 1704.5.1 item 2c
				b1. Test masonry units, mortar and grout (unit strength method). OR	T	Lab	2105.2.2.1.2
				b2. Test masonry prisms (prism test method).	T	Lab	2105.2.2.2 ASTM C1314.
				d. Verify proportions of site-prepared, premixed or preblended mortar and grout	P	SI	Table 1704.5.1, item 1 ASTM C780.
				e. Test core-drilled samples.	T	Lab	2105.3
				f. Inspect preparation of prisms or specimens.	C	SI	Table 1704.5.1 item 5. ASTM C1314.
				g. Verify size, location and condition of all dowels, construction supporting masonry, etc.	P	SI	
			X	**h.** Verify specified size, grade, and type of reinforcement.	P	SI	
				i. Welding of reinforcing steel	Provide special inspection per STEEL below.		
			X	**j.** Inspect placement of reinforcement, cleanliness of grout space, connectors, masonry units, and construction of mortar joints. "Wet-sticking" of bars is not permitted	P	SI	Table 1704.5.1 item 4
			X	k. Verify protection of masonry during cold weather (temperature below 40º F) or hot weather (temperature above 90º).	P	SI	Table 1704.5.1 item 2e, 2104.3, 2104.4
			X	**l.** Inspect type, size, and location of anchors and all other items to be embedded in masonry including other details of anchorage of masonry to structural members, frames and other construction	P	SI	Table 1704.5.1 item 2b
			X	**m. Inspect grout placement to ensure compliance with Code and construction document provisions. Verify lap lengths are provided and not located within cold joints. Observe/verify proper vibration and consolidation of grout.**	C	SI	Table 1704.5.1 item 4
		2. VENEER		**a.** Verify proportions of site-prepared mortar and grout and/or verify certification of premixed mortar.	P	SI	ASTM C780.
				b. Inspect placement of units and construction of mortar joints.	P	SI	
				c. Inspect placement of reinforcement, connectors and anchors.	P	SI	
				d. Inspect type, size, and location of anchors and all other items to be embedded in masonry including details of anchorage of masonry to structural members, frames and other construction	P	PI	1405.5, 1405.6., 1405.9
				e. Verify protection of masonry during cold weather (temperature below 40º F) or hot weather (above 90º).	P	SI	2104.3, 2104.4
				f. Test veneer bond strength	T	Lab	Field constructed mock-up laboratory tested in accordance with ASTM C482
	X	3. POST-INSTALLED ANCHORS IN MASONRY	X	**a.** Inspect installation of post-installed anchors	C	SI	As required by hardware's ICC Report (ESR) indicated on the construction drawings.

MATERIAL	APPLICABLE TO THIS PROJECT	COMPONENT	REQUIRED	TEST OR SPECIAL INSPECTION	TYPE	PERFORMED BY	CODE REFERENCE AND NOTES
D. STEEL (1704.3, see Exeptions')	X	1. STRUCTURAL STEEL AND COLD-FORMED STEEL USED FOR STRUCTURAL PURPOSES	X	**a.** Verify that all materials are appropriately marked and that: • Mill certificates indicate material properties that comply with requirements, • Material sizes, types and grades comply with requirements.	P	SI or PI	1708.4. By special inspector when performed off-site; by project inspector for steel shipped directly to project site without welding or fabrication.
			X	**b.** Test unidentified materials	T	Lab	2203.1. ASTM A370.
			X	**c.** Verify member locations, bracing and all details constructed in the field.	P	SI	Table 1704.3 item 6
			X	**d.** Verify stiffener locations, connection tab locations and all construction details fabricated in the shop.	P	SI	Table 1704.3 item 6
	X	2. HIGH STRENGTH BOLTS	X	**a.** Verify identification markings and manufacturer's certificates of compliance conform to ASTM standards specified in the approved documents. While work is in progress, verify parts, installation, tightening, calibration, and sequencing procedures.	P	SI	1704.3.3
				b. Test high-strength bolts, nuts and washers.	T	Lab	ASTM F606., A370.
			X	**c.** Bearing-type ("snug tight") connections: Drawn together and properly snugged	P	SI	1704.3.3.1
			X	**d.** Slip-critical connections.	P & C	SI	"Continuous" or "Periodic" depends on the tightening method used, see 1704.3.3.2 and 1704.3.3.3
	X	3. ALL WELDING	X	**a.** Verify weld filler material identification markings per AWS designation listed on the approved documents and the WPS.	P	SI	1704.3.1
			X	**b.** Verify weld filler material manufacturer's certificate of compliance.	P	SI	1704.4
			X	**c.** Verify WPS, PQR's (as required) welder qualifications and equipment.	P	SI	1704.3.1
	X	4 SHOP WELDING	X	**a.** Inspect groove, multi-pass, and fillet welds > 5/16".	C	SI	Per AISC 360 (and AISC 341 as applicable). 1707.1.2.
			X	**b.** Inspect single-pass fillet welds ≤ 5/16"	P	SI	Per AISC 360 (and AISC 341 as applicable). 1704.3.2.2.1
			X	**c.** Inspect welding of stairs and railing systems.	P	SI	1704.3.2.2.5
			X	**d.** Verification of reinforcing steel weldability	P	SI	1704.4.1; verify carbon equivalent reported on mill certificates.
				e. Inspect welding of reinforcing steel in concrete moment frames and boundaries & shear reinforcement of special concrete shear walls.	C	SI	1704.3.1.3, 1704.3.1.4 and Table 1704.3 Item 5b. AWS D1.4.
				f. Inspect welding of all other reinforcing steel.	P	SI	Table 1704.3
	X	5. FIELD WELDING	X	**a.** Inspect groove, multi-pass, and fillet welds > 5/16". Observe field fit-up'	C	SI	Per AISC 360 (and AISC 341 as applicable).
			X	**b.** Inspect single-pass fillet welds ≤ 5/16"	P	SI	Per AISC 360 (and AISC 341 as applicable).
			X	**c.** Inspect end-welded studs (ASTM A-108) installation (including bend test)	P	SI	Per AISC 360. 1704.3.2.2.3
			X	**d.** Inspect floor and roof deck welds	P	SI	Per AISC 360. 1704.3.2.2.2
			X	**e.** Inspect welding of structural cold-formed steel	P	SI	* May be performed by the project inspector when approved by the EOR. 1704.3.1.2 and 1704.3.1.4
			X	**f.** Inspect welding of stairs and railing systems	P	SI	1704.3.2.2.5
			X	**g.** Verification of reinforcing steel weldability	P	SI	1704.4.1; verify carbon equivalent reported on mill certificates.
				h. Inspect welding of reinforcing steel in concrete moment frames and boundaries & shear reinforcement of special concrete shear walls.	C	SI	1704.3.1.3, 1704.3.1.4 and Table 1704.3 Item 5b. AWS D1.4.
				i. Inspect welding of all other reinforcing steel.	P	SI	Table 1704.3
	X	6. NON-DESTRUCTIVE TESTING	X	**a.** Ultrasonic: Moment frames and CJP (cantilever) moment connections	T	Lab	AISC 341, App. Q 5.2. AWS D1.1, D1.8.- ANSI/ASNT CP-189, SNT-TC-1A. - ASTM E543, E1212
				b. Magnetic Particle	T	Lab	
	X	7. STEEL JOISTS AND TRUSSES	X	**a.** Verify size, type and grade for all chord and web members as well as connectors and weld filler material; verify joist profile, dimensions and camber (if applicable); verify all weld locations, lengths, spacing, and profiles; mark or tag each joist.	P	SI	
		8. SPRAY-APPLIED FIRE PROOFING		**a.** Examine structural steel surface conditions, inspect application, take samples, measure thickness, and verify compliance of all aspects of application with approved documents.	C	Special	1704.11 ASTM E605. Spec. Section 78100
				b. Test bond strength.	T	Lab	ASTM E736.
				c. Test density.	T	Lab	ASTM E605.
	X	9. STEEL FABRICATION SHOP	X	**a.** Verify fabricator's, qualifications, fabrication and quality control procedures.	P	SI	1704.2.1, unless as allowed in accordance with 1704.2.2
				b. Verify all aspects of shop fabrication including member locations, dimensional layout of all parts and pieces, all welding, bolting, etc.	C	SI	1704.2

LEGEND

1 Type -	**2 Performed By** -
C = Continuous – Indicates that a continuous special inspection is required	**GE** – Indicates that the special inspection is to be performed by a registered geotechnical engineer or his or her authorized representative
P = Periodic – Indicates that a periodic special inspection is required	**Lab** – Indicates that the test is to be performed by a testing laboratory approved by the Building Official and/or the Engineer of Record.
T = Test – Indicates that a test is required	**PI** – Indicates that the special inspection is to be performed by the Special Inspector of Record (Project Inspector).
	SI – Indicates that the special inspection is to be performed by a special inspector

CONTRACTOR OBLIGATIONS FOR SPECIAL INSPECTIONS:

1. CONTRACTOR SHALL FURNISH AND MAINTAIN AN "INSPECTION AND TESTING REGISTER" AT THE FIELD OFFICE. CONTRACTOR SHALL DIRECT EVERY SPECIAL INSPECTOR OR TESTING AGENCY PERSONNEL TO SIGN-IN AT THE REGISTER BEFORE COMMENCING THEIR INSPECTION OR SAMPLING. AT THE CONCLUSION OF THE DAY'S INSPECTION OR SAMPLING THE INSPECTOR OR TESTING AGENCY PERSONNEL SHALL SIGN-OUT.
2. CONTRACTOR SHALL MAINTAIN A LOG OF ALL AREAS OF CONSTRUCTION APPROVED TO DATE FOR EACH TRADE. NO FURTHER CONSTRUCTION SHALL TAKE PLACE OVER AREAS NOT APPROVED BY THE SPECIAL INSPECTOR. NO CONSTRUCTION PROGRESS SHALL TAKE PLACE WITHOUT ADDRESSING THE DEFICIENCIES OR DISCREPANCIES BROUGHT TO THE ATTENTION OF THE CONTRACTOR BY THE SPECIAL INSPECTOR OR BY THE ENGINEER. CONSTRUCTION THAT WERE NOT APPROVED BY THE SPECIAL INSPECTOR OR BY THE ENGINEER IS SUBJECT TO DEMOLITION AND REPLACEMENT WITH PROPER MODIFICATIONS AT THE CONTRACTOR'S EXPENSE.
3. INDIVIDUALS APPROVED BY THE ENGINEER AS SPECIAL INSPECTOR SHALL HAVE COMPLETE ACCESS TO THE JOB SITE AT ALL TIMES DURING WHICH THE CONSTRUCTION IS TAKING PLACE. CONTRACTOR SHALL PROVIDE NECESSARY WALKWAYS, ELEVATORS, PLATFORMS, LIFTS FOR PROPER INSPECTION OF THE COMPLETED OR ON-GOING CONSTRUCTION.
4. CONTRACTOR SHALL GIVE ADEQUATE NOTICE TO THE SPECIAL INSPECTOR PER HIS/HER SCHEDULE AND REQUEST FOR HIS/HER AVAILABILITY ON THE DAY OF CONSTRUCTION THAT REQUIRES CONTINUOUS INSPECTION. BUILDING CONSTRUCTION REQUIRING CONTINUOUS ON-SITE INSPECTION SHALL NOT TAKE PLACE WITHOUT THE PRESENCE OF THE SPECIAL INSPECTOR. CONSTRUCTION NOT COMPLYING WITH THIS PROVISION AND THAT IS NOT IN COMPLIANCE WITH THE QUALITY CONTROL REQUIREMENTS SHALL BE REPLACED AT NO COST TO THE OWNER OR HIS AGENTS.
5. CONTRACTOR SHALL MAKE AVAILABLE, AT ALL TIMES, ACCESS TO THE SPECIAL INSPECTOR AND TESTING AGENCIES, ALL CONTRACT DOCUMENTS INCLUDING PLANS, SPECIFICATIONS, SHOP DRAWINGS, ADDENDA; CHANGE ORDERS, FIELD ORDERS ETC.

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University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
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ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

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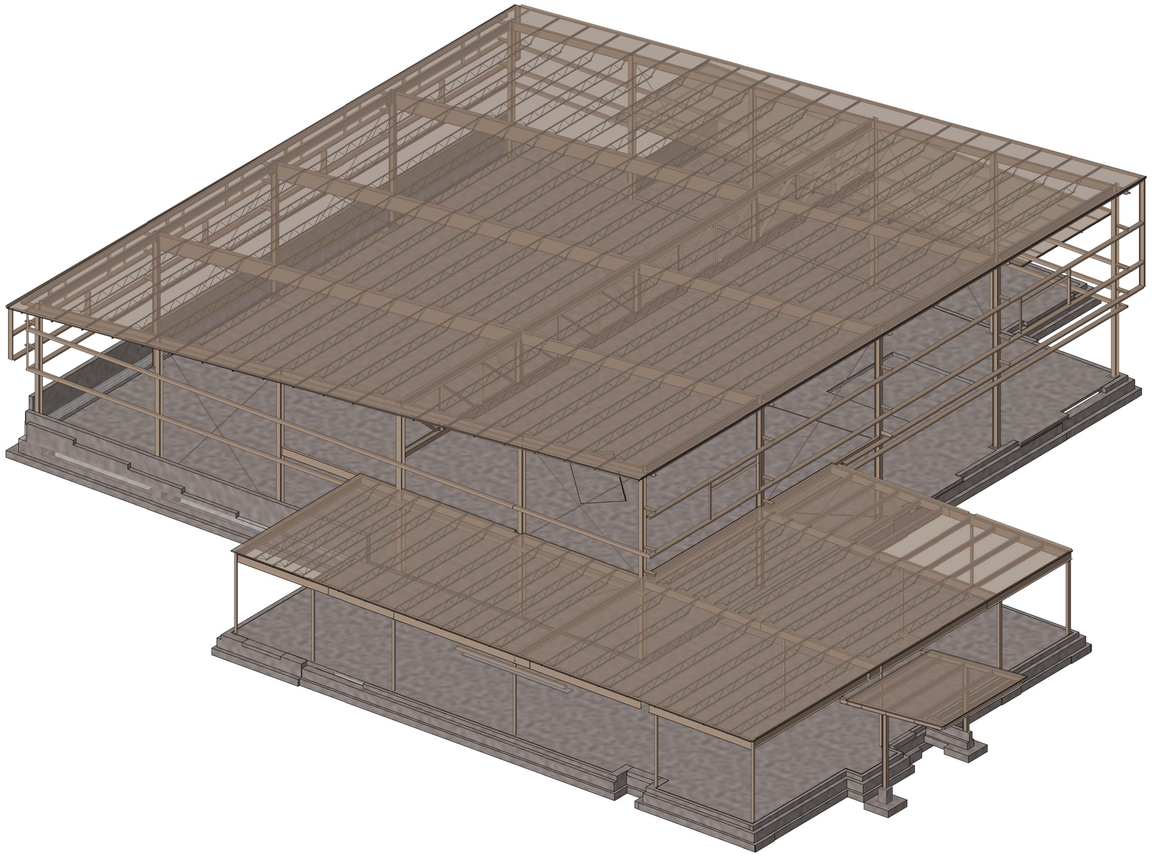
GENERAL NOTES - SPECIAL INSPECTIONS
DRAWING NO. S0.1 | BID PKG.

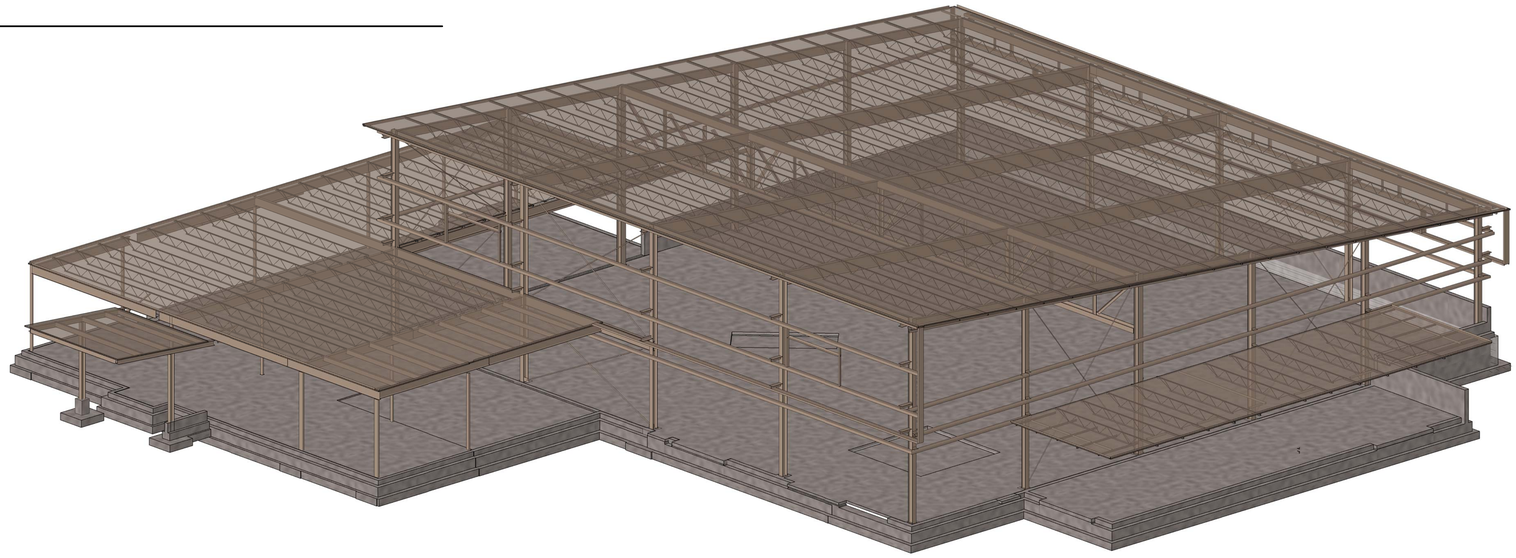
CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY



1 HITTING PAVILION FRAMING

SCALE:



2 HITTING PAVILION FRAMING

SCALE:

CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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DATE	5/30/12
	HITTING PAVILION 3D VIEWS
DRAWING NO.	H-S1.0
BID PKG.	

CONTRACTOR NOTE:

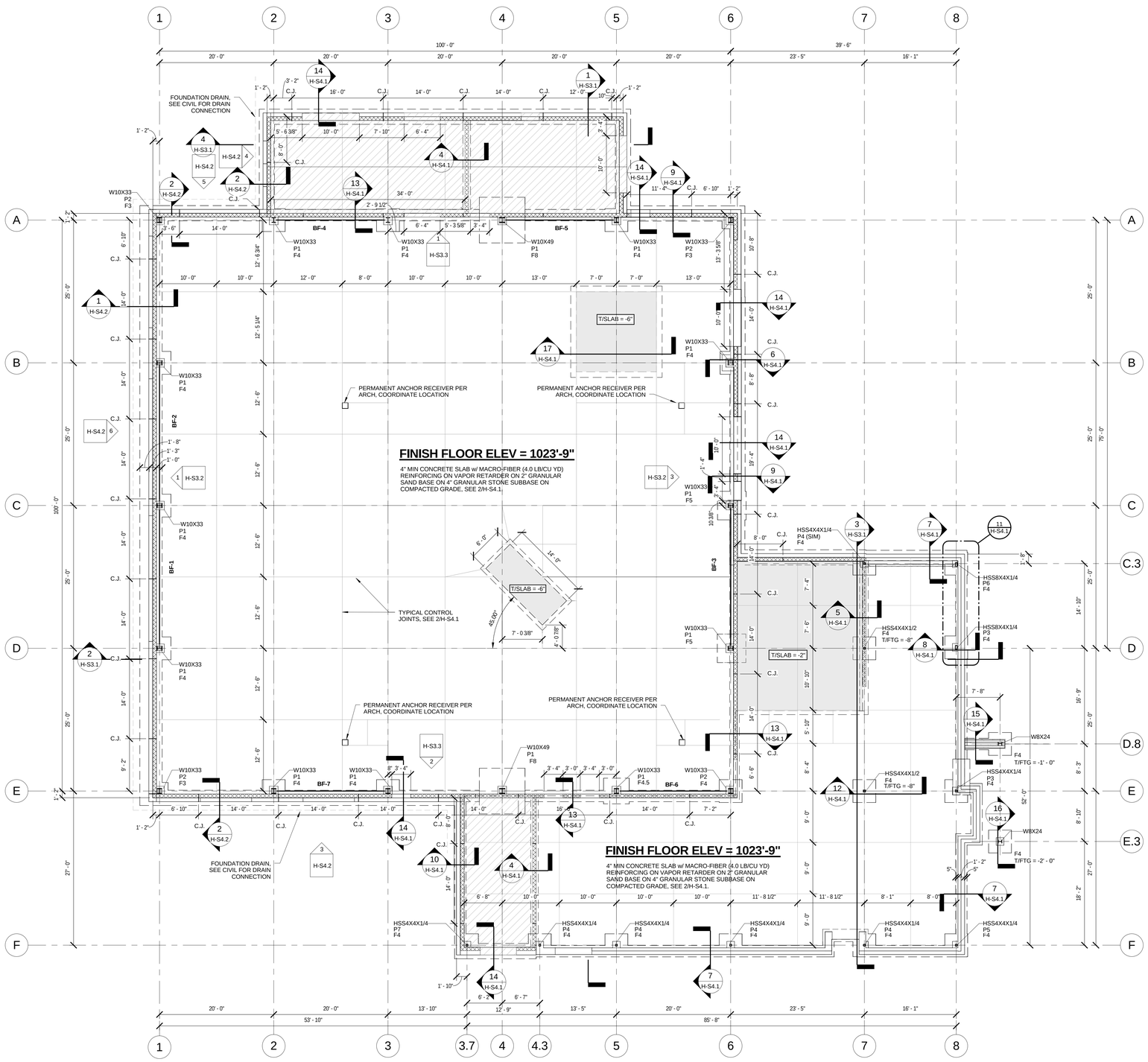
NOTE THAT THE EXISTING UTILITIES INDICATED ON THE SITE SURVEY REFLECT THE MOST CURRENT SITE INFORMATION AVAILABLE, HOWEVER THERE MAY BE VARIOUS EXISTING ACTIVE UTILITIES BELOW GRADE THAT ARE NOT REFLECTED ON THE SURVEY. CARE SHALL BE TAKEN BY THE CONSTRUCTION TEAM DURING EXCAVATION PROCEDURES TO PROTECT ALL EXISTING UTILITIES.

HITTING PAVILION FOOTING SCHEDULE

MARK	FOOTING SIZE Length	Width	Depth	Reinforcing
F3	3' - 0"	3' - 0"	1' - 0"	4) #4 BARS EW BOTTOM
F4	4' - 0"	4' - 0"	1' - 0"	6) #4 BARS EW BOTTOM
F4.5	4' - 6"	4' - 6"	1' - 0"	6) #4 BARS E.W. BOTTOM
F5	5' - 0"	5' - 0"	1' - 0"	5) #5 BARS EW BOTTOM
F8	8' - 0"	8' - 0"	1' - 6"	8) #6 BARS E.W. BOTTOM

LEGEND

- INDICATES DEPRESSED FLOOR SLAB REGION, SEE PLAN FOR DEPTH OF DEPRESSION.
- 6" MIN CONCRETE SLAB w/ MACRO-FIBER (4.0 LB/CU YD) REINFORCING ON VAPOR RETARDER ON 2" GRANULAR BASE ON 4" GRANULAR STONE SUBBASE ON GRADE, SEE 2/H-S4.1.
- INDICATES 8" CMU WALL PER SCHEDULE
- INDICATES CONRETE CURB/WALL



PLAN NORTH

HITTING PAVILION FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

FOUNDATION NOTES:

1. THE FOUNDATIONS HAVE BEEN DESIGNED BASED ON RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL INVESTIGATION REPORT PREPARED BY AMEC, DATED APRIL 2012.
2. FOUNDATIONS SHALL BEAR ON PROPERLY PREPARED AND COMPACTED SOILS AS OUTLINED IN THE GEOTECHNICAL REPORT REFERENCED ABOVE.
3. PROTECT EXISTING UTILITIES AND STRUCTURES (OVERHEAD OR UNDERGROUND) WITHIN THE WORK AREA AS WELL AS ANY EXISTING FOUNDATION SYSTEM(S). THE CONTRACTOR IS RESPONSIBLE FOR ANY SHORING OR BRACING OF SUCH COMPONENTS.
4. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE CONCRETE OR GROUT HAS ATTAINED FULL DESIGN STRENGTH. CONTRACTOR SHALL BRACE OR PROTECT ALL BUILDINGS AND WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED FULL DESIGN STRENGTH. CONTRACTOR SHALL PROVIDE FOR DESIGN, PERMITS (AS REQ'D) AND INSTALLATION OF SUCH BRACING.
5. FOUNDATIONS SHALL BE PLACED ACCORDING TO THE DEPTHS SHOWN ON THE DRAWINGS. SHOULD SOIL ENCOUNTERED AT THESE DEPTHS NOT BE APPROVED BY THE INSPECTOR OR SOILS ENGINEER, FOUNDATION ELEVATIONS/DIMENSIONS MAY NEED TO BE MODIFIED BY THE ENGINEER. NOTIFY THE ENGINEER OF RECORD IF THIS IS THE CASE.
6. REFERENCE ELEVATION 0'-0" = ABSOLUTE ELEVATION 1023.75' AS SHOWN ON THE CIVIL DRAWINGS.
7. TOP OF PIER = 0'-0" UNLESS NOTED OTHERWISE.
   TOP OF FOOTING = -2'-0" UNLESS NOTED OTHERWISE.
8. SEE ARCHITECTURE DRAWINGS FOR NON-LOAD BEARING METAL STUD PARTITION WALLS. SEE 4/H-S4.1 & 5/H-S4.1 FOR THICKENED SLAB @ CMU PARTITION WALLS.
9. SEE SHEET H-S4.3 FOR TYPICAL MASONRY DETAILS. SEE S0.0 FOR LINTEL SCHEDULE.

DESIGN CRITERIA - HITTING PAVILION:

DEAD LOAD:

ROOF DECKING	= 3 PSF
ROOF FRAMING SELF-WEIGHT	= 4 PSF
INSULATION	= 3 PSF
COLLATERAL	= 10 PSF
TOTAL	= 20 PSF

LIVE LOAD:

ROOF	= 20 PSF

SNOW LOAD:

GROUND SNOW LOAD	= 15 PSF
FLAT ROOF SNOW LOAD	= 9.45 PSF
SNOW DRIFT LOADING	= SEE SNOW DRIFT DIAGRAMS ON H-S2.2

CMU WALL REINFORCEMENT SCHEDULE - HITTING PAVILION

WALLS	VERTICAL REINFORCING	LAP LENGTH	BOND BEAMS	JOINTS
8" STRUCTURAL WALLS (AS SHOWN HEREIN)	#4 AT 48" OC TYP	36"	8'-0" OC MAX, TOP & BOTTOM OF WALL, REINFORCED w/ 2) #5	LADDER TYPE WL7 (9GA.) GALVANIZED WELDED WIRE JOINT REINFORCING AT 16" OC

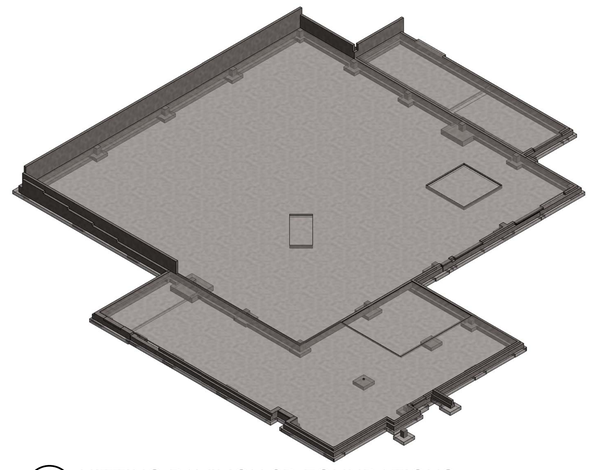
1. PROVIDE 2) #5 VERTICAL REINFORCEMENT AT JAMBS OF EACH OPENING, AT BOTH SIDES OF CONTROL JOINT; AND AT EACH END OR CORNER OF WALL, UNLESS SHOWN OTHERWISE.
2. SEE H-S4.3 FOR TYPICAL MASONRY CONSTRUCTION DETAILS.
3. PROVIDE DOWELS AT FOUNDATION TO MATCH VERTICAL REINFORCING.
4. CONDUIT, DATA, ETC. SHALL BE COORDINATED SUCH THAT THEY OCCUR IN UNREINFORCED VERT. CELLS ONLY. REINFORCING BAR LOCATIONS SHALL HAVE PRECEDENCE. DO NOT CUT REINFORCING BARS.
5. ALL STEEL REINFORCEMENT SHALL BE CONTINUOUS. PROVIDE PROPER LAP SPLICES PER 12/H-S4.3 AS REQ'D. LAP SPLICES SHALL NOT BE LOCATED BETWEEN GROUT LIFTS/CONSTRUCTION JOINTS.
6. VERTICAL REINFORCING SHALL BE GROUTED SOLID WITHIN AN 8" CELL MIN, UNLESS NOTED OTHERWISE.

COLUMN / BASEPLATE / ANCHOR BOLT SCHEDULE				
COLUMN SIZE	BASEPLATE (t x L x W)		ANCHOR BOLT SIZE	EMBEDMENT
W10x33	3/4" x 12" x 10"	BP-1	4) 3/4" Ø	12"
W10x49	3/4" x 12" x 12"	BP-2	4) 3/4" Ø	12"
HSS 4 x 4 x 1/4	3/4" x 10" x 10"	BP-3	4) 3/4" Ø	12"
HSS 4 x 4 x 1/2	3/4" x 10" x 10"	BP-3	4) 3/4" Ø	9"
HSS 8 x 4 x 1/4	3/4" x 14" x 10"	BP-4	4) 3/4" Ø	12"
W8x24	3/4" x 14" x 14"	BP-5	4) 3/4" Ø	9"

NOTE: SEE BASEPLATE - BP-* DETAILS ON H-S4.5 FOR BASEPLATE DETAILS

HITTING PAVILION PIER SCHEDULE				
MARK	SIZE	REINFORCING VERTICAL	REINFORCING TIES	T/PIER ELEV.
P1	1'-6" x 2'-0"	6) #6	#4 @ 8" OC	0'-0"
P2	2'-0" x 2'-0"	8) #6	#4 @ 8" OC	0'-0"
P3	1'-4" x 2'-1 5/8"	6) #6	#3 @ 12" OC	-0'-8"
P4	1'-4" x 2'-3 1/2"	6) #6	#3 @ 12" OC	-0'-8"
P5	2'-3 1/2" x 2'-1 5/8"	8) #6	#3 @ 12" OC	-0'-8"
P6	2'-4" x 2'-1 5/8"	8) #6	#3 @ 12" OC	-0'-8"
P7	2'-3 1/2" x 2'-6"	8) #7	#3 @ 12" OC	-0'-8"

NOTE: SEE PIER - P* DETAILS ON H-S4.1 FOR REINFORCING DETAILS



2 HITTING PAVILION 3D FOUNDATIONS

SCALE:

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HITTING PAVILION FOUNDATION PLAN

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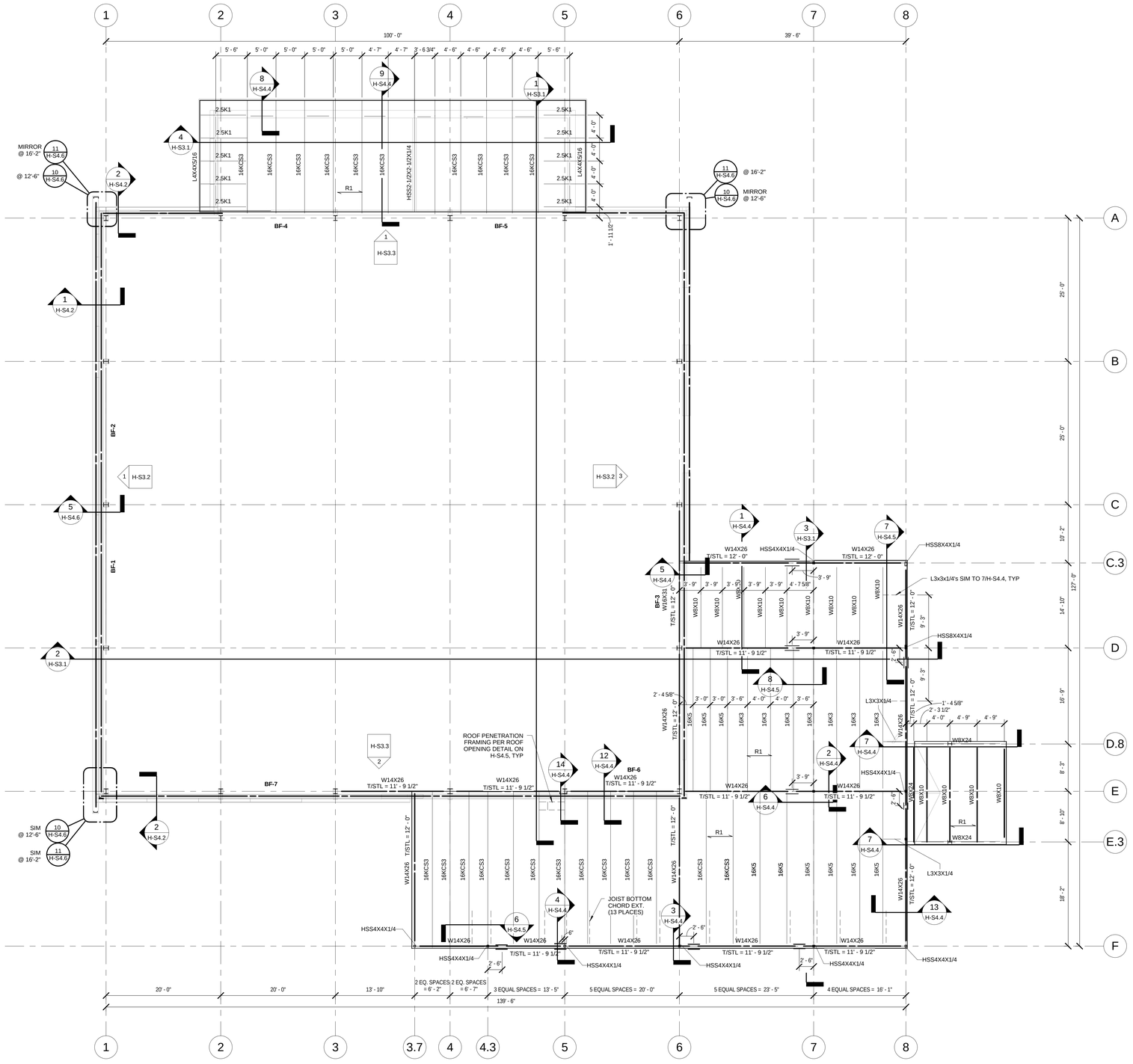


PLAN NORTH

HITTING PAVILION - LOW ROOF & INTERMEDIATE FRAMING PLAN

SCALE: 1/8" = 1'-0"

FRAMING NOTES:

1. REFER TO S0.0 FOR STRUCTURAL GENERAL NOTES.
2. R1 - DENOTES 1.5B22 ROOF DECK.
   FASTENING: SUPPORT: #12 TEK SCREWS @ 12" OC
   SIDELAPS: #10 TEK SCREWS @ 12" OC

DESIGN CRITERIA - HITTING PAVILION:

DEAD LOAD:

ROOF DECKING	= 3 PSF
ROOF FRAMING SELF-WEIGHT	= 4 PSF
INSULATION	= 3 PSF
COLLATERAL	= 10 PSF
TOTAL	= 20 PSF

LIVE LOAD:

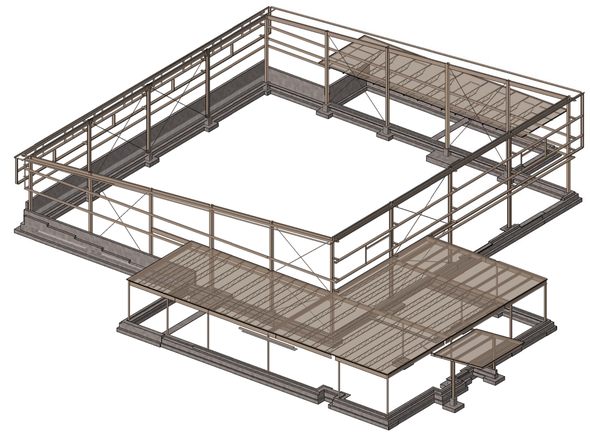
ROOF	= 20 PSF

SNOW LOAD:

GROUND SNOW LOAD	= 15 PSF
FLAT ROOF SNOW LOAD	= 9.45 PSF
SNOW DRIFT LOADING	= SEE SNOW DRIFT DIAGRAMS ON H-S2.2

LEGEND

INDICATES BEAM SPLICE LOCATION



2 HITTING PAVILION LOW ROOF & INTERMEDIATE FRAMING

SCALE:

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HITTING PAVILION LOW ROOF & INTERMEDIATE FRAMING PLAN

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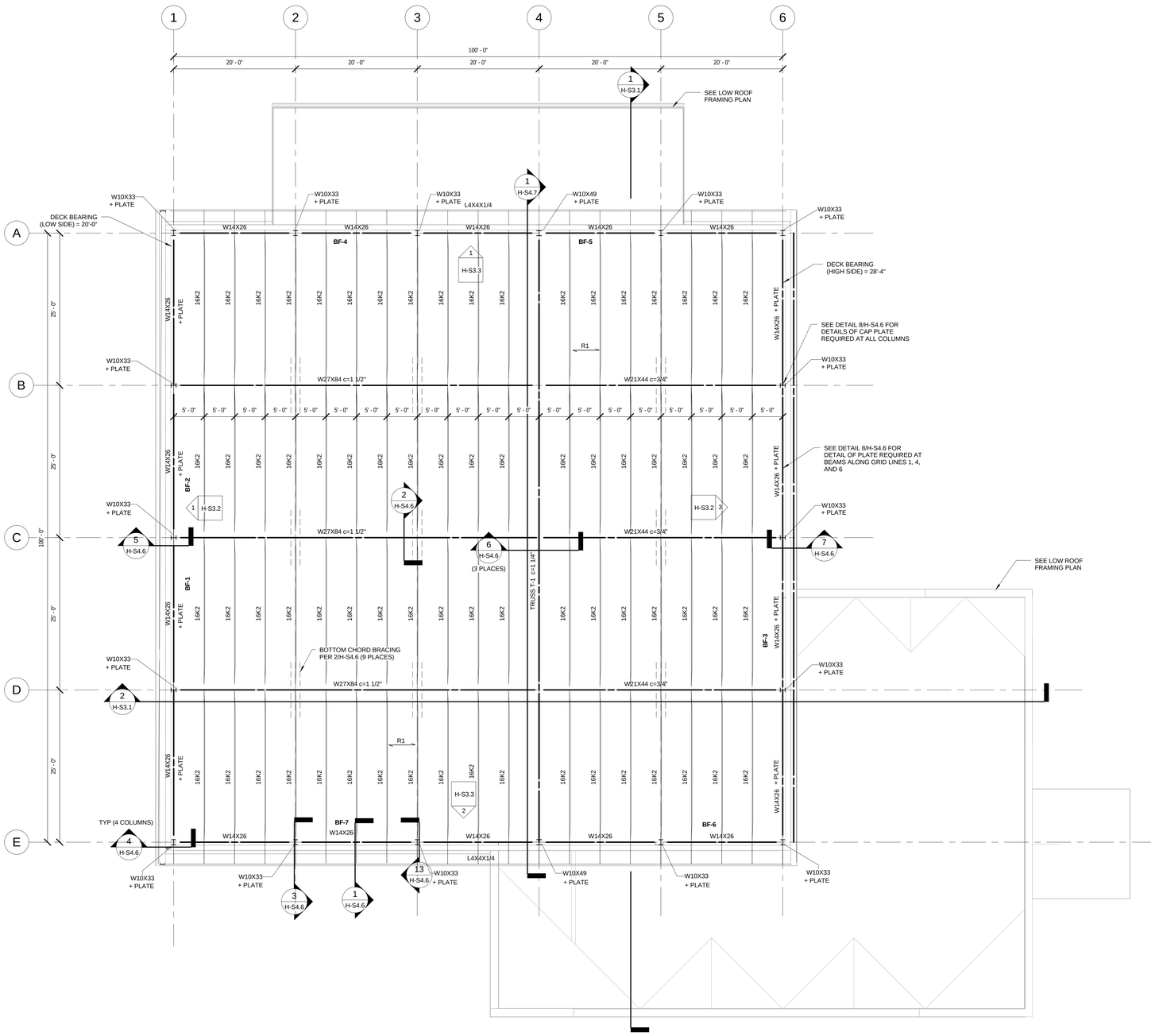


PLAN NORTH

HITTING PAVILION ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

FRAMING NOTES:

1. REFER TO S0.0 FOR STRUCTURAL GENERAL NOTES.
2. R1 - DENOTES 1.5B22 ROOF DECK.
   FASTENING: SUPPORT: #12 TEK SCREWS @ 12" OC
   SIDELAPS: #10 TEK SCREWS @ 12" OC
3. JOISTS SHALL BE DESIGNED FOR NET WIND UPLIFT OF 16 PSF (NOT INCLUDING JOIST SELF WEIGHT.)
4. JOIST MANUFACTURER SHALL PROVIDE SJI STANDARD BRIDGING TO RESIST UPLIFT AND PROVIDE NECESSARY STABILITY DURING ERECTION.

DESIGN CRITERIA - HITTING PAVILION:

DEAD LOAD:

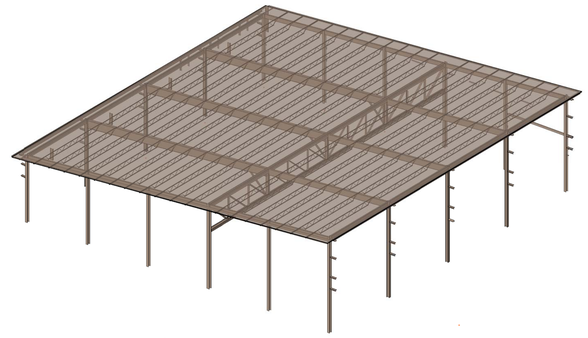
ROOF DECKING	= 3 PSF
ROOF FRAMING SELF-WEIGHT	= 4 PSF
INSULATION	= 3 PSF
COLLATERAL	= 10 PSF
TOTAL	= 20 PSF

LIVE LOAD:

ROOF	= 20 PSF

SNOW LOAD:

GROUND SNOW LOAD	= 15 PSF
FLAT ROOF SNOW LOAD	= 9.45 PSF
SNOW DRIFT LOADING	= SEE SNOW DRIFT DIAGRAMS ON H-S2.2



3 HITTING PAVILION HIGH ROOF FRAMING

SCALE:

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SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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HITTING PAVILION HIGH ROOF FRAMING PLAN

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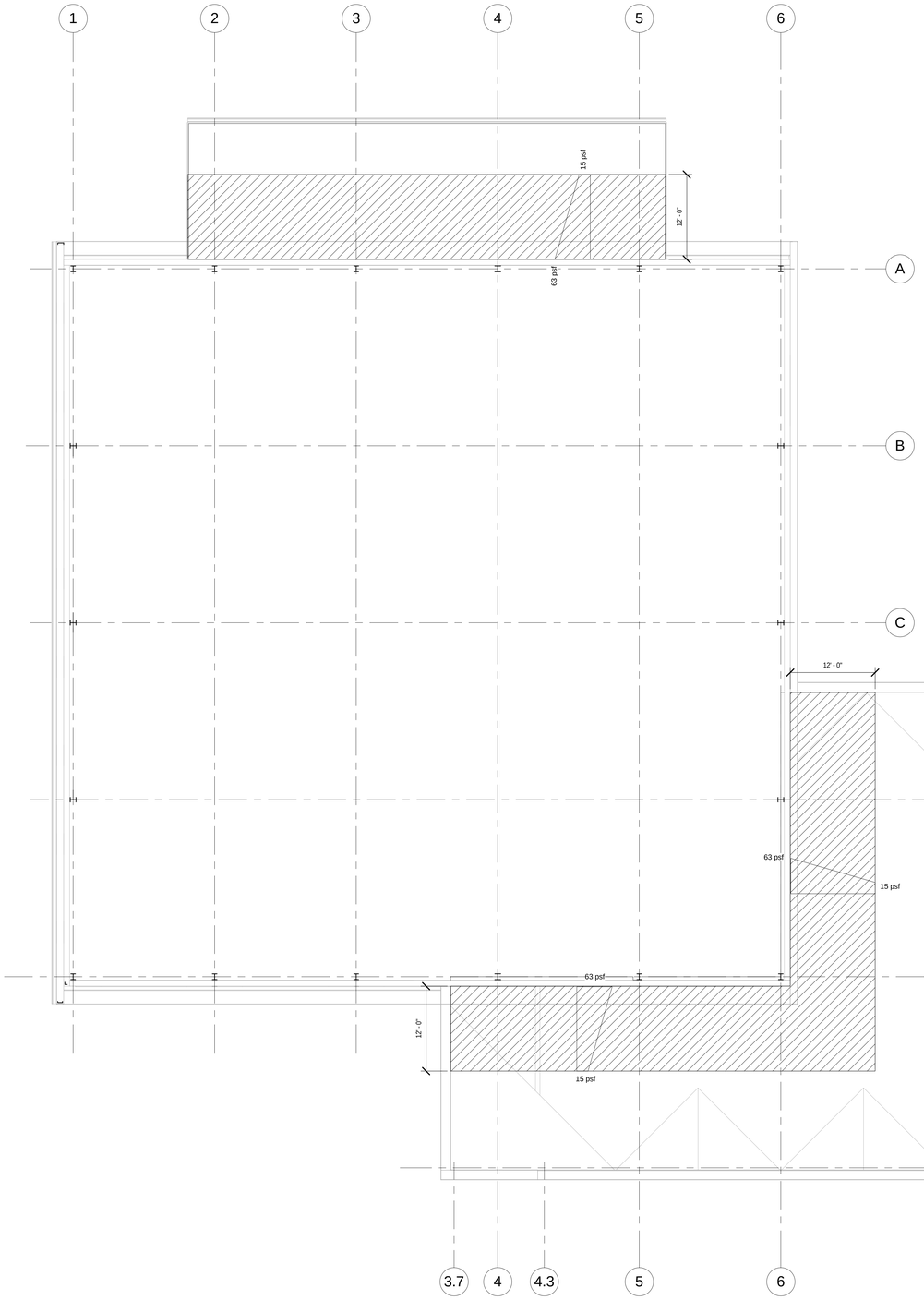
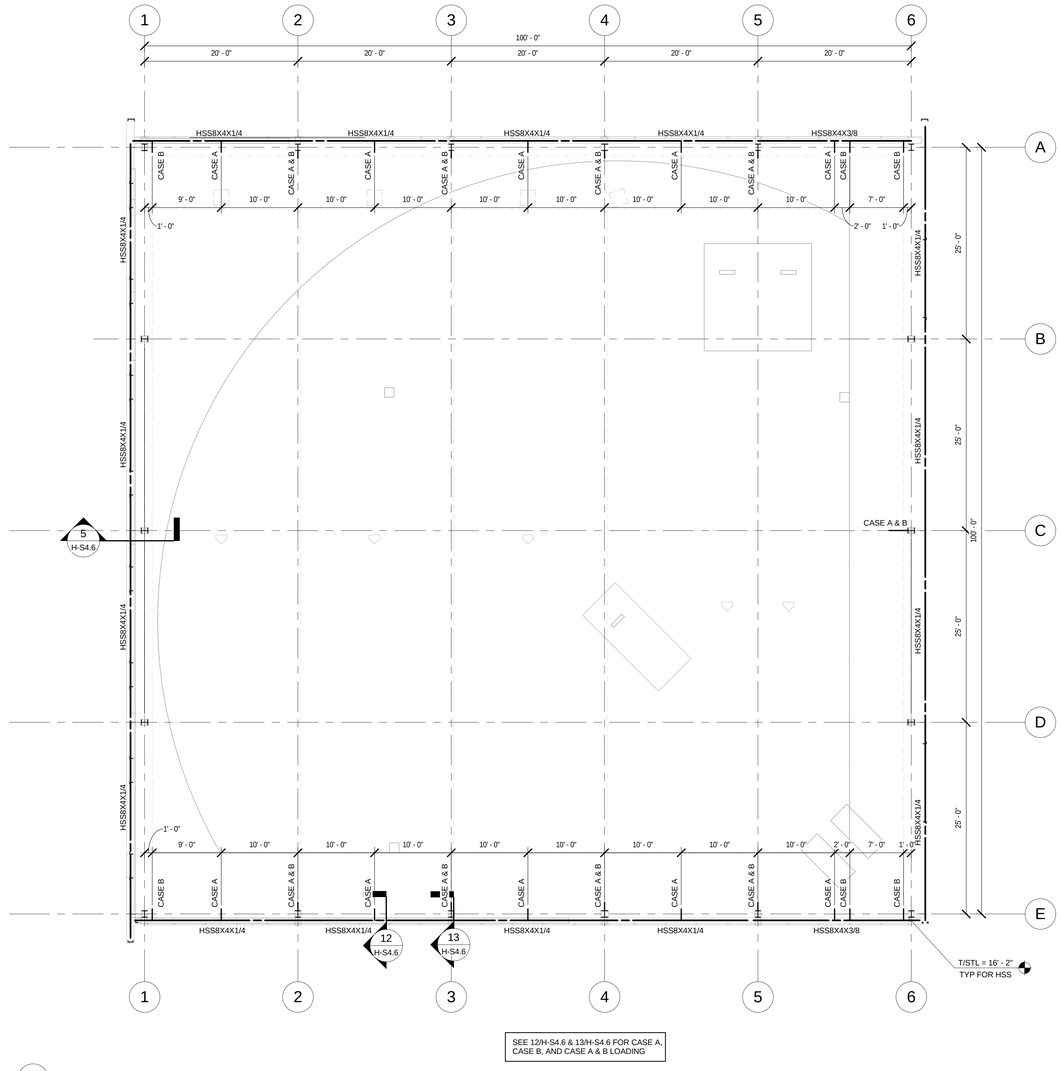


PLAN NORTH

HITTING PAVILION SNOW DRIFT PLAN

SCALE: 1/8" = 1'-0"



SEE 12/H-S4.6 & 13/H-S4.6 FOR CASE A, CASE B, AND CASE A & B LOADING

PLAN NORTH

HITTING PAVILION - NET FRAMING PLAN

SCALE: 1/8" = 1'-0"

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HITTING PAVILION LOADING PLANS

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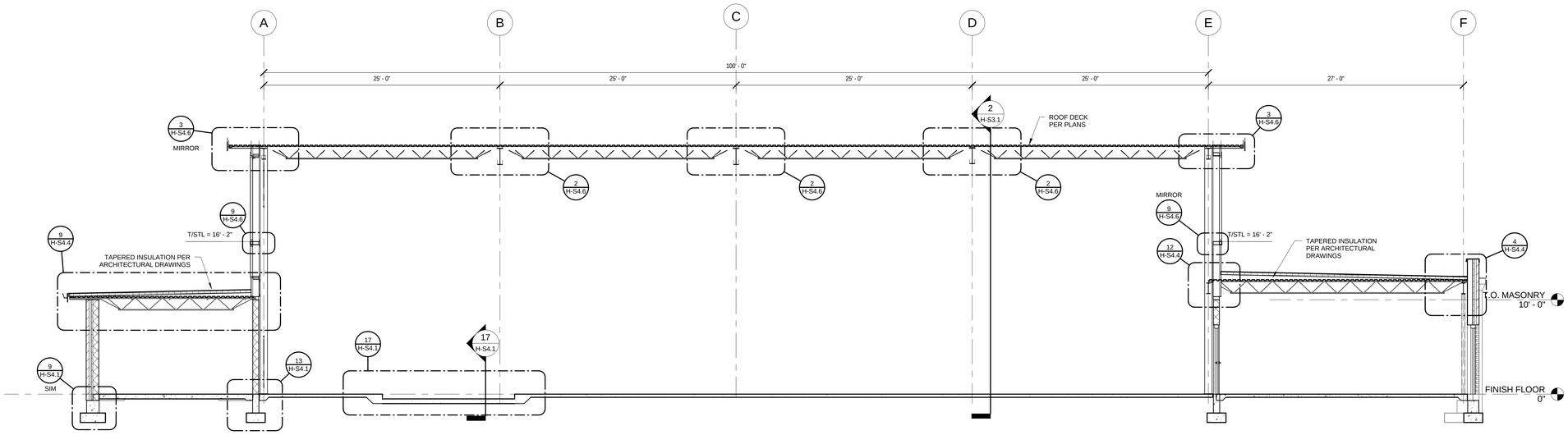
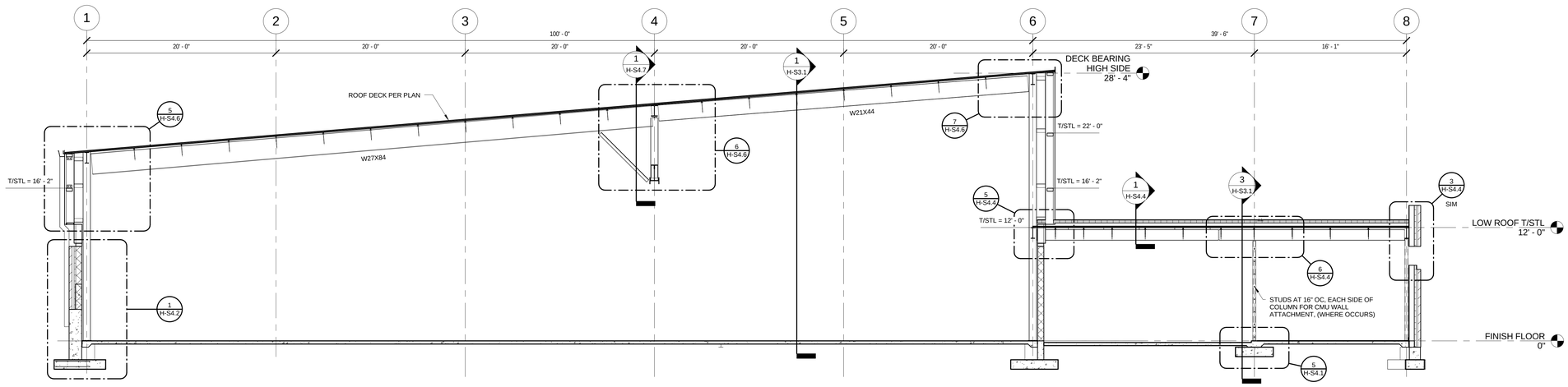


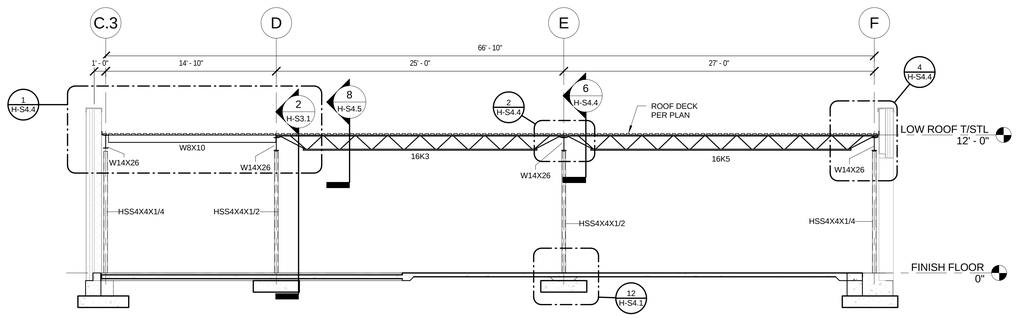
1 BUILDING SECTION - EAST/WEST

H-S1.1 SCALE: 3/16" = 1'-0"



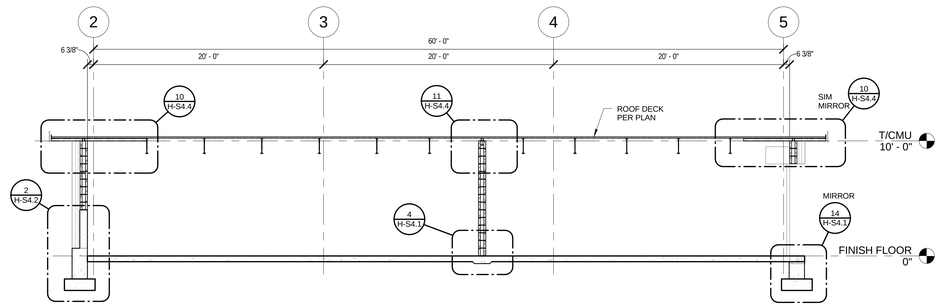
2 BUILDING SECTION - NORTH/SOUTH

H-S1.1 SCALE: 3/16" = 1'-0"



3 BUILDING SECTION - EAST/WEST LOCKER ROOM AREA

H-S1.1 SCALE: 3/16" = 1'-0"



4 STORAGE SECTION

H-S1.1 SCALE: 3/16" = 1'-0"

CONSTRUCTION DOCUMENTS

0 1/2" 1"

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HITTING PAVILION
BUILDING SECTIONS

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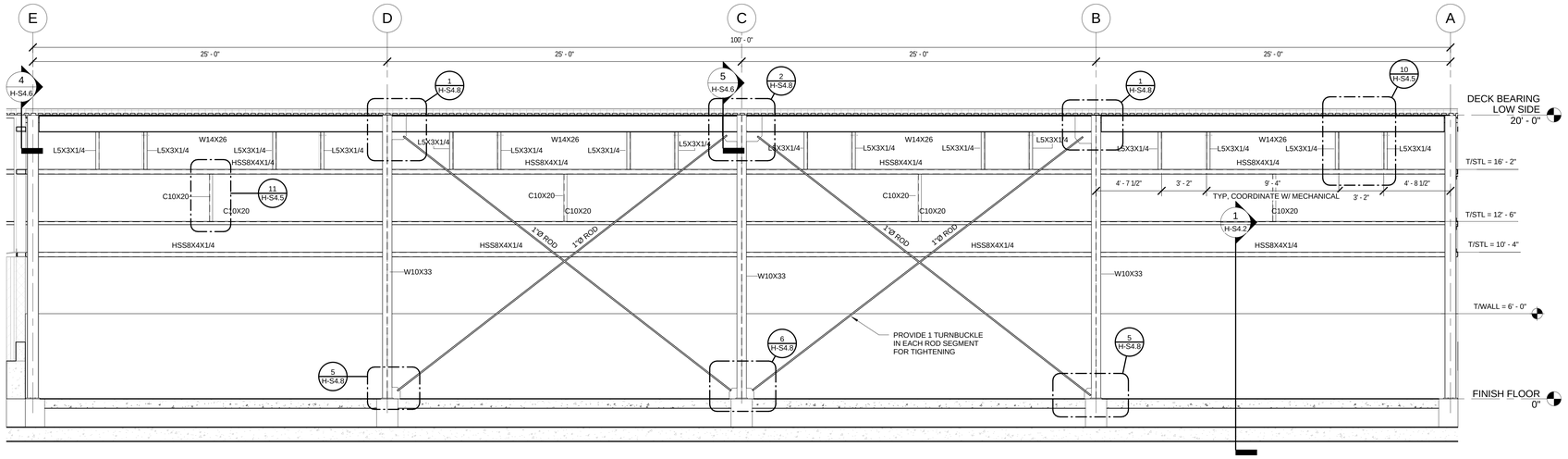
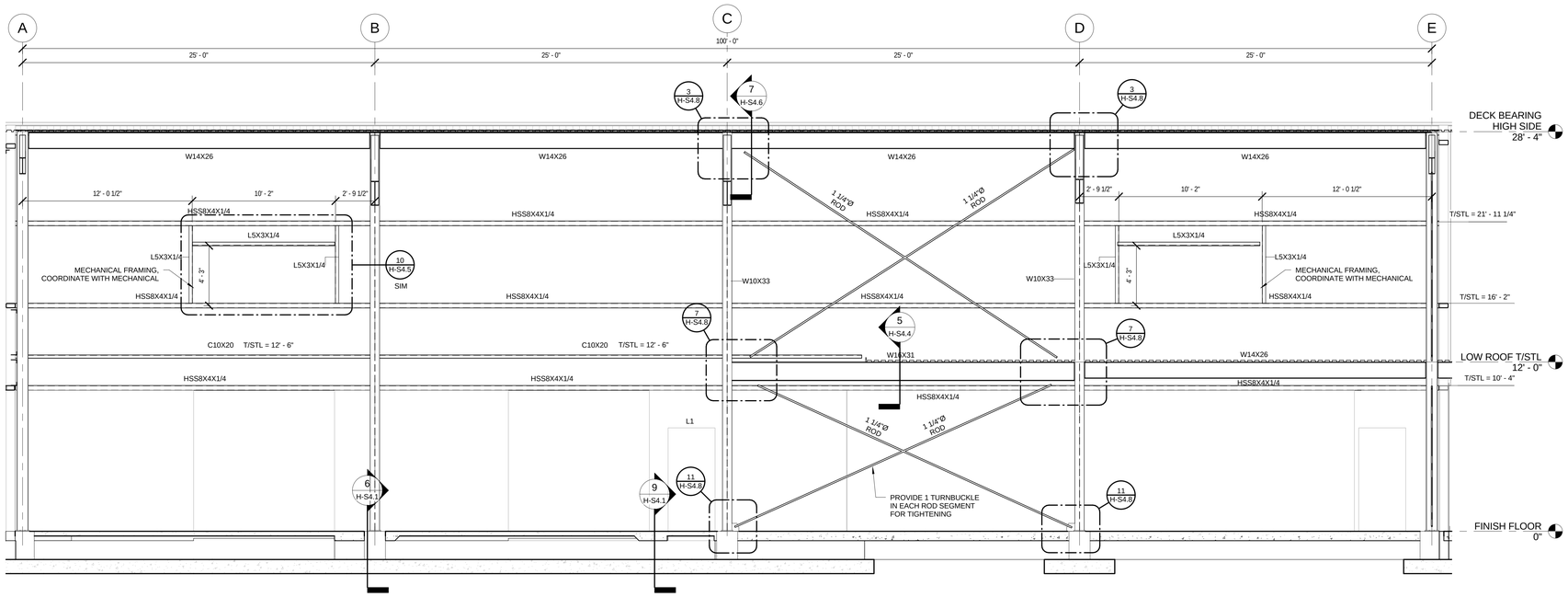


1 BF-1 & BF-2 - X-BRACING AT COLUMN LINE '1'

H-S1.1 SCALE: 1/4" = 1'-0"



3 BF-3 - X-BRACING AT COLUMN LINE '6'

H-S1.1 SCALE: 1/4" = 1'-0"

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ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

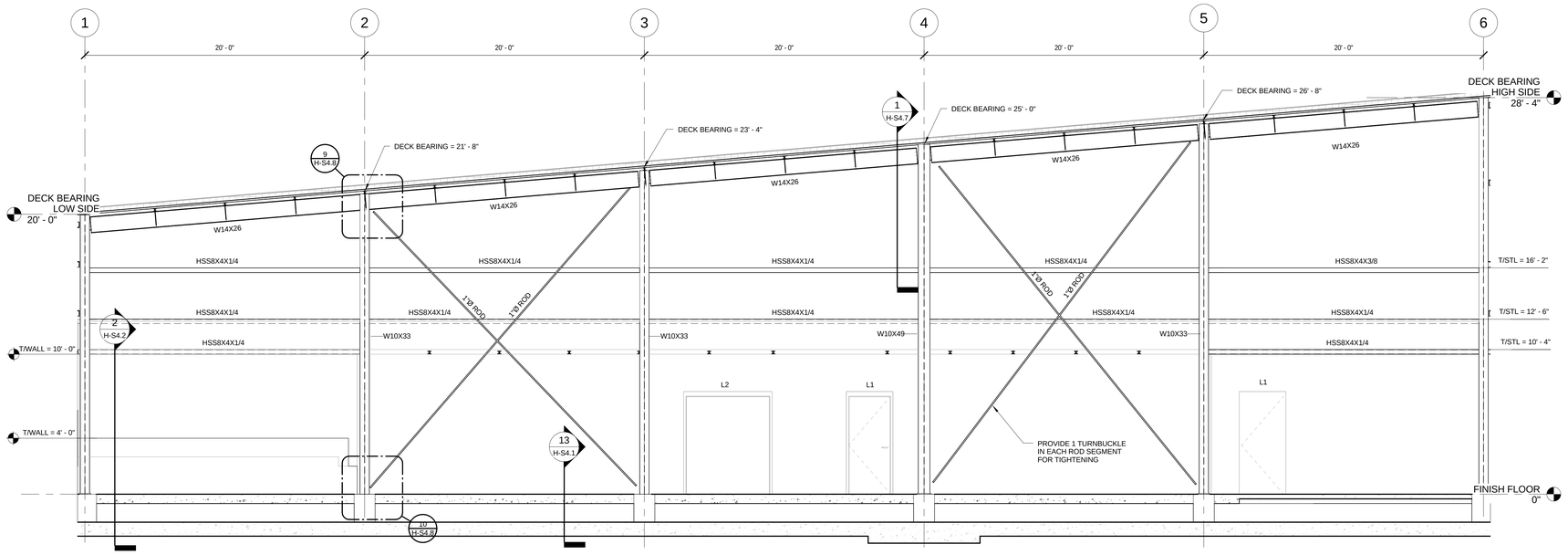
REVISIONS		
#	Description	Date

COMM. NO. 2012.030.00
MADE BY HRG | CHECKED
DATE 5/30/12
HITTING PAVILION
BRACING
ELEVATIONS
DRAWING NO. H-S3.2 | BID PKG.

CONSTRUCTION DOCUMENTS

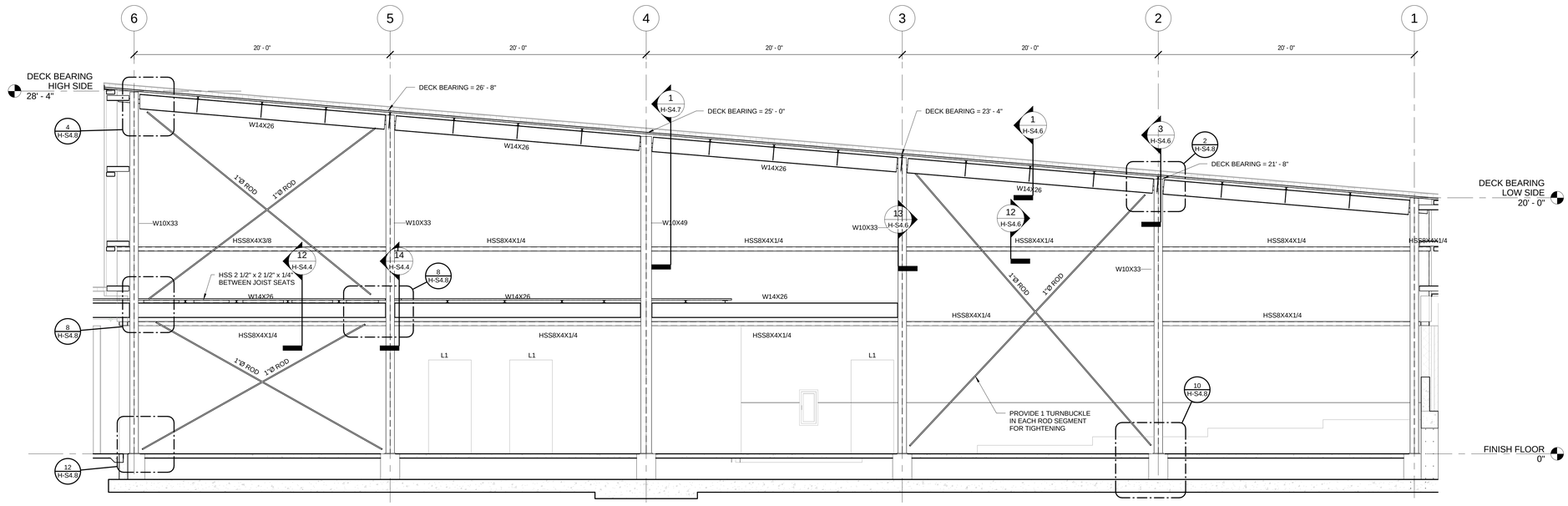
SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY 0 1/2" 1"

5/30/2012 3:09:03 PM
C:\Users\hgeibel\Documents\2012030HP_STRUCT_hgeibel.rvt



1 BF-4 & BF-5 - X-BRACING AT COLUMN LINE 'A'

SCALE: 1/4" = 1'-0"



2 BF-6 & BF-7 - X-BRACING AT COLUMN LINE 'E'

SCALE: 1/4" = 1'-0"

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University of Kentucky
Renovate/Upgrade Softball Complex

PROJECT NUMBER: 2338.0

SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

COMM. NO.	2012.030.00
MADE BY	HRG
CHECKED	
DATE	5/30/12

HITTING PAVILION
BRACING
ELEVATIONS

DRAWING NO. **H-S3.3**

BID PKG.

CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

0 1/2" 1"

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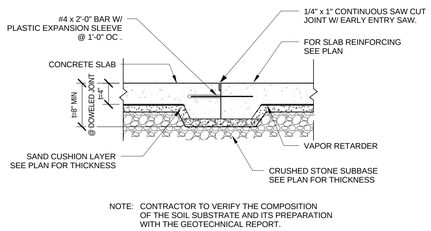
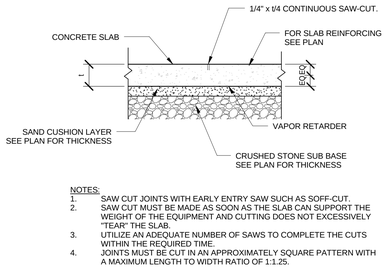


NOTE: CONTRACTOR TO VERIFY THE COMPOSITION OF THE SOIL SUBSTRATE AND ITS PREPARATION WITH THE GEOTECHNICAL REPORT.

1 SLAB ON GRADE CONSTRUCTION JOINT

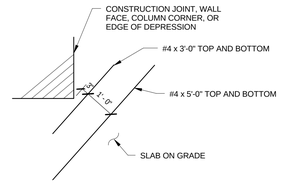
SCALE: 3/4" = 1'-0"



NOTES:
1. SAW CUT JOINTS WITH EARLY ENTRY SAW SUCH AS SOFF-CUT.
2. SAW CUT MUST BE MADE AS SOON AS THE SLAB CAN SUPPORT THE WEIGHT OF THE EQUIPMENT AND CUTTING DOES NOT EXCESSIVELY "TEAR" THE SLAB.
3. UTILIZE AN ADEQUATE NUMBER OF SAWS TO COMPLETE THE CUTS WITHIN THE REQUIRED TIME.
4. JOINTS MUST BE CUT IN AN APPROXIMATELY SQUARE PATTERN WITH A MAXIMUM LENGTH TO WIDTH RATIO OF 1:1.25.

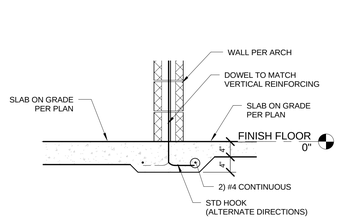
2 SLAB ON GRADE CONTROL JOINT

SCALE: 3/4" = 1'-0"



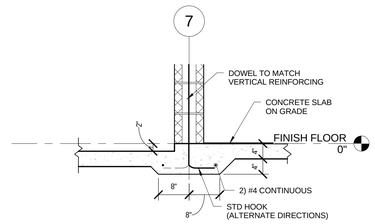
3 RE-ENTRY REINFORCING

SCALE: 1/2" = 1'-0"



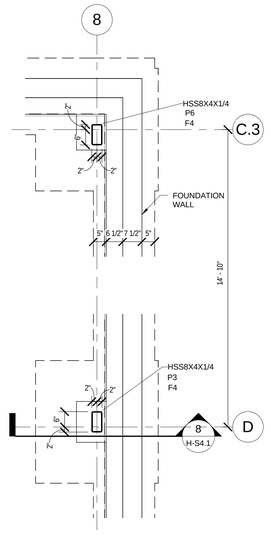
4 INTERIOR CMU FOOTING

H-S1.1 SCALE: 3/4" = 1'-0"



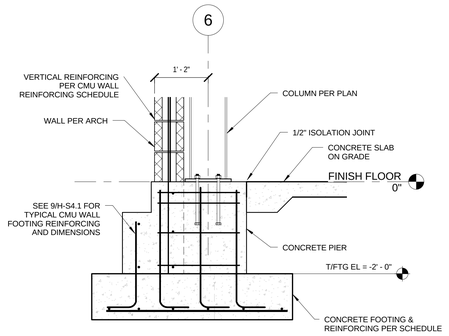
5 SLAB AT DEPRESSION

H-S1.1 SCALE: 3/4" = 1'-0"



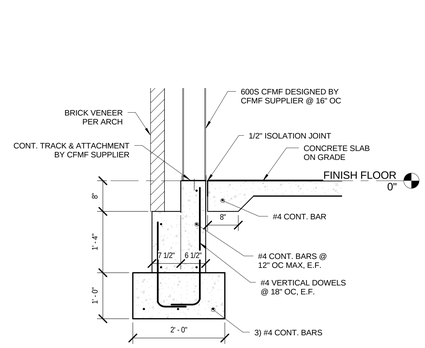
11 FRAME FOOTING

H-S1.1 SCALE: 1/2" = 1'-0"



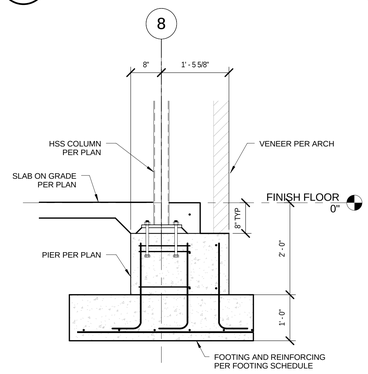
6 TYP. COLUMN FOOTING

H-S1.1 SCALE: 3/4" = 1'-0"



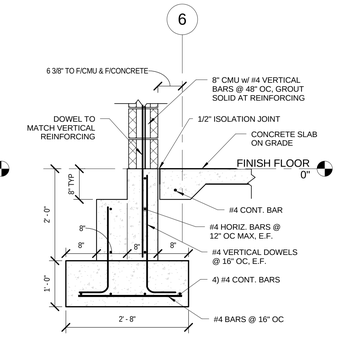
7 TYP. METAL STUD WALL FOOTING

H-S1.1 SCALE: 3/4" = 1'-0"



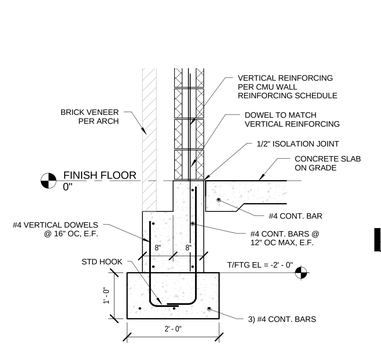
8 COLUMN FOOTING

H-S1.1 SCALE: 3/4" = 1'-0"



9 TYP. CMU WALL FOOTING

H-S1.1 SCALE: 3/4" = 1'-0"

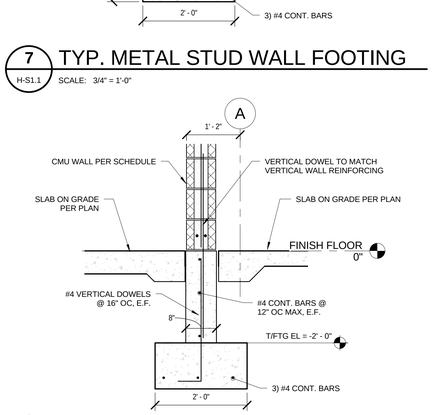


10 FOOTING AT FIRE PUMP

H-S1.1 SCALE: 3/4" = 1'-0"

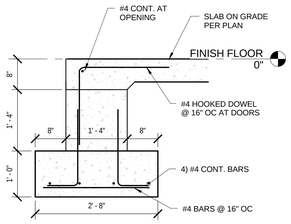
12 INTERIOR FOOTING

H-S1.1 SCALE: 3/4" = 1'-0"



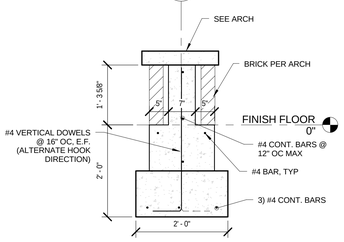
13 CMU WALL FOOTING

H-S1.1 SCALE: 3/4" = 1'-0"



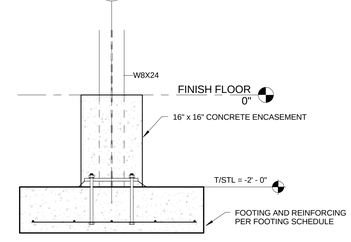
14 TYPICAL POUR-THRU

H-S1.1 SCALE: 3/4" = 1'-0"



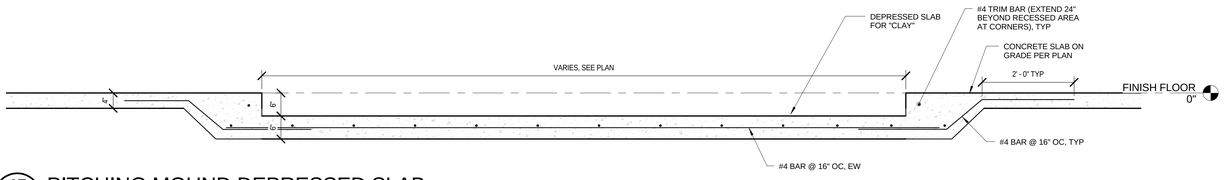
15 BENCH FOOTING

H-S1.1 SCALE: 3/4" = 1'-0"



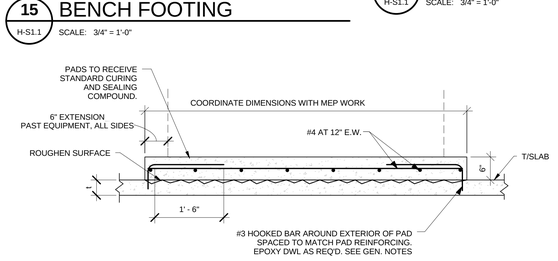
16 FRONT CANOPY COLUMN FOOTING

H-S1.1 SCALE: 3/4" = 1'-0"



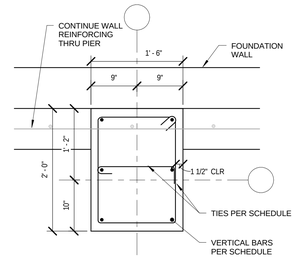
17 PITCHING MOUND DEPRESSED SLAB

H-S1.1 SCALE: 3/4" = 1'-0"



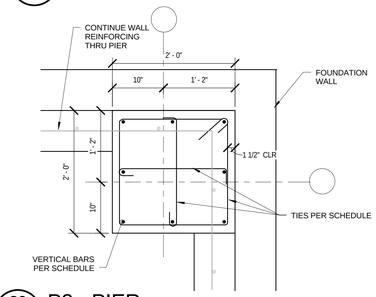
18 TYPICAL HOUSEKEEPING PAD -HP

SCALE: 3/4" = 1'-0"



19 P1 - PIER

SCALE: 1" = 1'-0"



20 P2 - PIER

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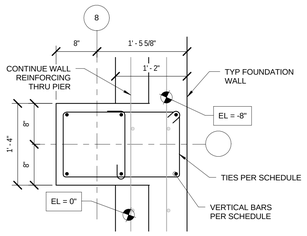

21 P3 - PIER

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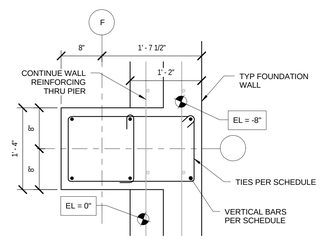

22 P4 - PIER

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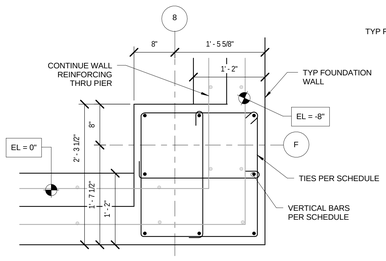

23 P5 - PIER

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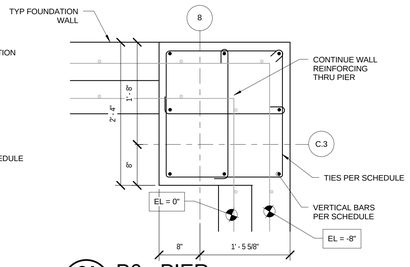

24 P6 - PIER

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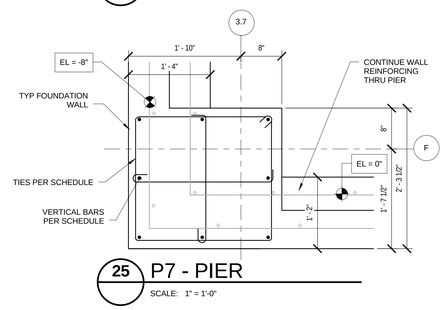

25 P7 - PIER

SCALE: 1" = 1'-0"

CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

0 1/2" 1"

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COMMONWEALTH OF KENTUCKY KYLE S. BESSLER 23151 LICENSED PROFESSIONAL ENGINEER

University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

COMM. NO.	2012.030.00
MADE BY	HRG
CHECKED	
DATE	5/30/12

HITTING PAVILION FOUNDATION SECTIONS & DETAILS

DRAWING NO. H-S4.1 BID PKG.

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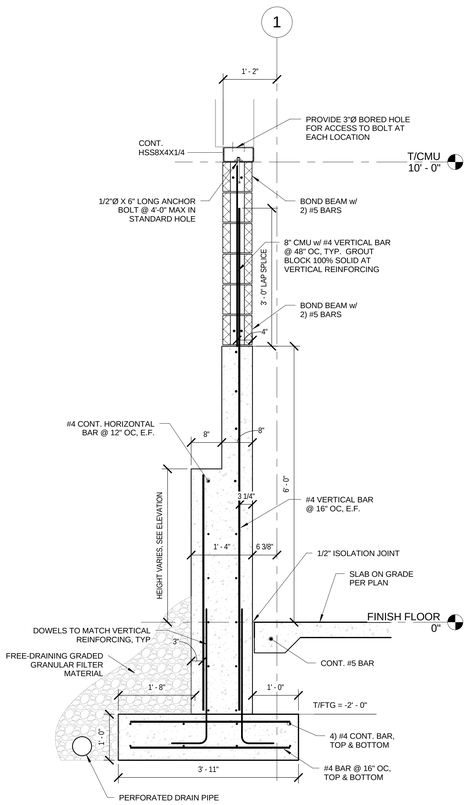
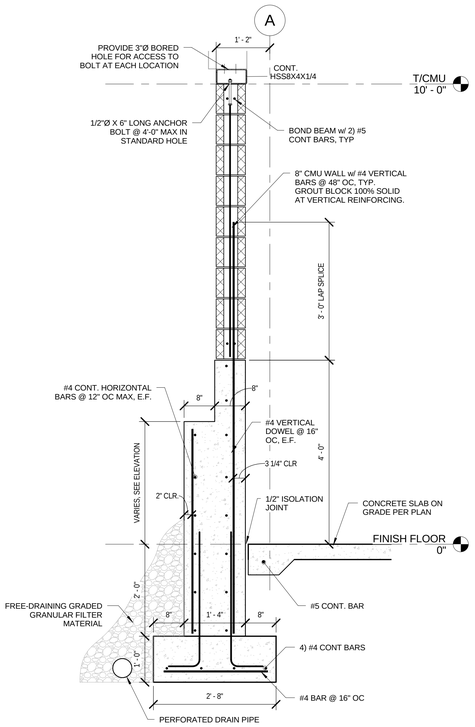
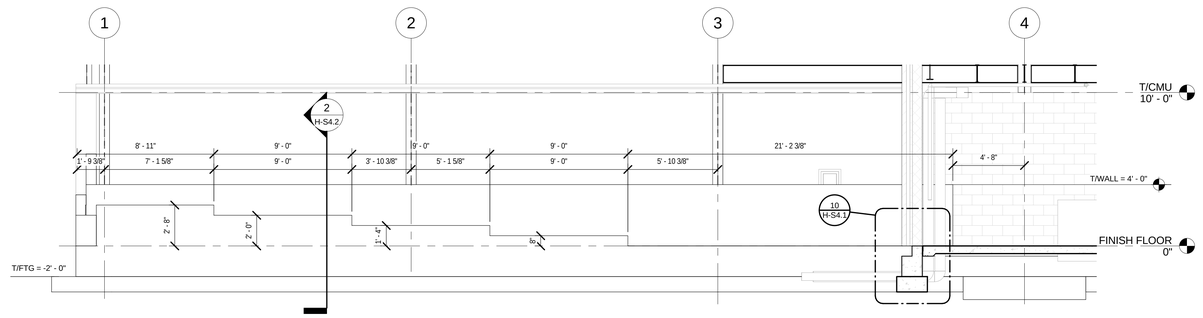


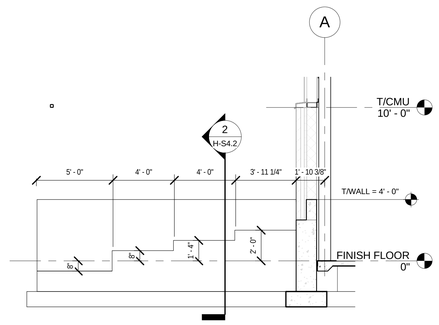
1 RETAINING WALL TO 6'-0"
H-S1.1 SCALE: 3/4" = 1'-0"



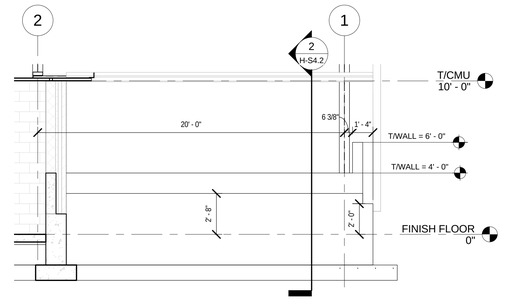
2 RETAINING WALL TO 4'-0"
H-S1.1 SCALE: 3/4" = 1'-0"



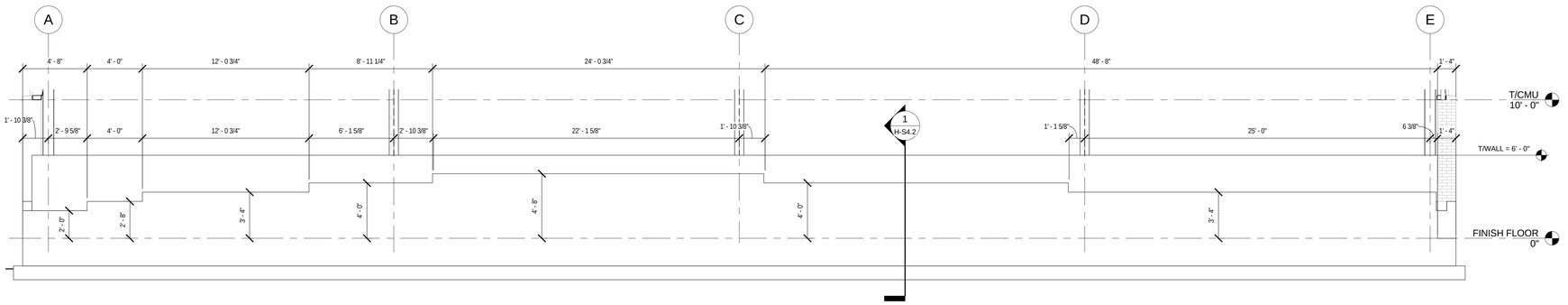
3 CONCRETE BRICK LEDGE STEPS AT COLUMN LINE 'E'
H-S1.1 SCALE: 1/4" = 1'-0"



4 CONCRETE BRICK LEDGE STEPS AT COLUMN LINE '2'
H-S1.1 SCALE: 1/4" = 1'-0"



5 CONCRETE BRICK LEDGE STEPS AT COLUMN LINE 'A'
H-S1.1 SCALE: 1/4" = 1'-0"



6 CONCRETE BRICK LEDGE STEPS AT COLUMN LINE '1'
H-S1.1 SCALE: 1/4" = 1'-0"

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University of Kentucky
Renovate/Upgrade Softball Complex
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REVISIONS		
#	Description	Date

COMM. NO. 2012.030.00
MADE BY HRG | CHECKED
DATE 5/30/12
FOUNDATION RETAINING WALL ELEVATIONS
DRAWING NO. H-S4.2 | BID PKG.

CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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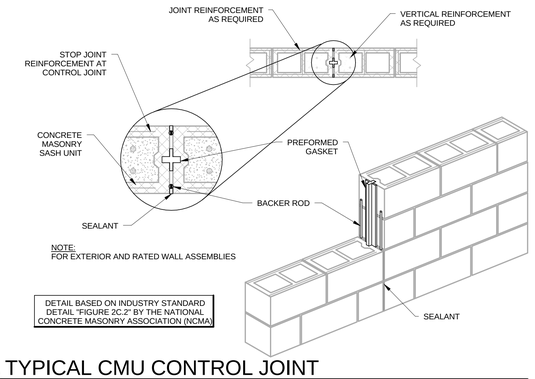
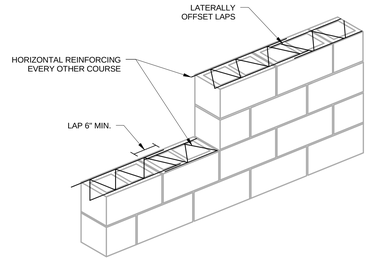


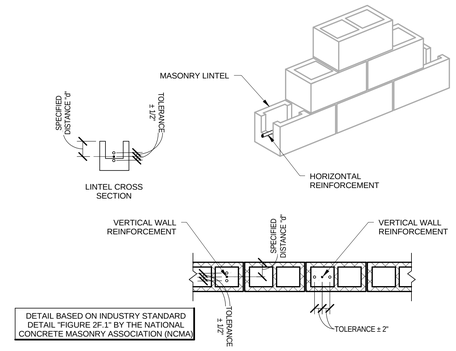
1 TYPICAL CMU CONTROL JOINT

SCALE: 3/4" = 1'-0"



2 HORIZONTAL JOINT REINFORCEMENT

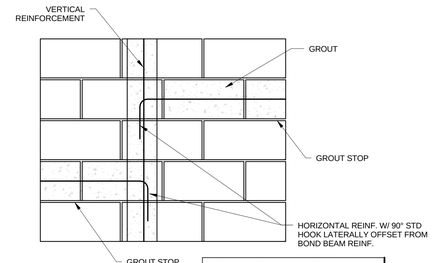
SCALE: 3/4" = 1'-0"



NOTE: REFER TO CMU LINTEL SCHEDULE FOR REINFORCEMENT SIZE.

3 CMU REINFORCEMENT TOLERANCE

SCALE: 3/4" = 1'-0"

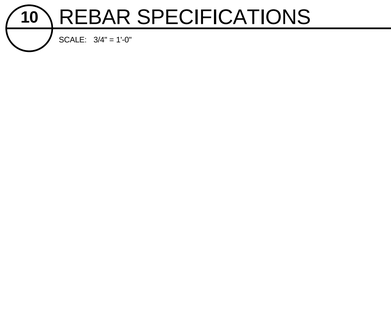


4 INTERSECTION OF REINF. AT BOND BEAM

SCALE: 1" = 1'-0"

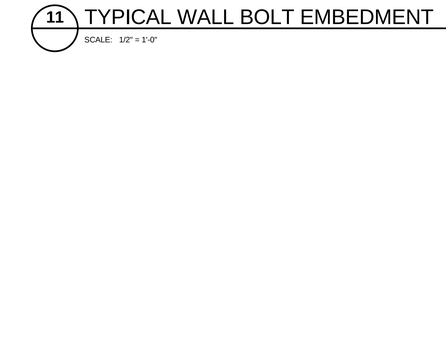
5 CMU CORNER REINFORCEMENT

SCALE: 1" = 1'-0"



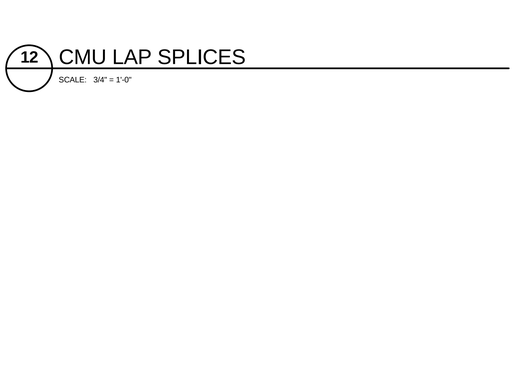
6 INTERSECTION OF REINF. AT T/WALL

SCALE: 1" = 1'-0"



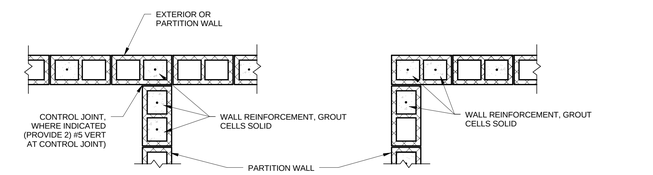
7 CMU REINFORCEMENT BEND TOLERANCE

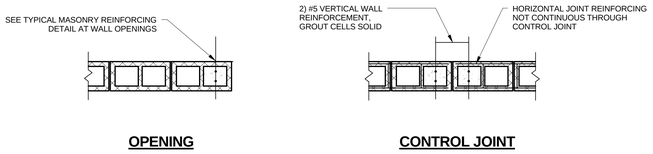
SCALE: 1" = 1'-0"



8 BED JOINT REINFORCEMENT

SCALE: 1" = 1'-0"



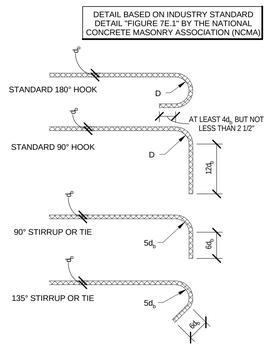


SEE PLAN AND SCHEDULE FOR REINFORCING

9 MASONRY VERTICAL REINFORCING

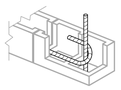
SCALE: 3/4" = 1'-0"

DETAIL BASED ON INDUSTRY STANDARD DETAIL "FIGURE 7E.1" BY THE NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA)



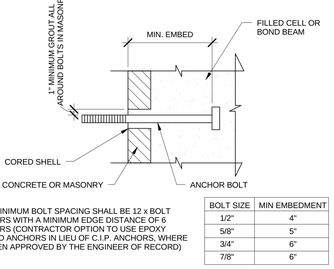
BAR SIZE AND TYPE	MINIMUM DIAMETER OF BEND, D[A]
NO.3 THROUGH NO. 7 GRADE 40	5 BAR DIAMETERS
NO. 3 THROUGH NO. 8 GRADE 50 OR GRADE 60	6 BAR DIAMETERS
NO. 9, NO. 10, AND NO. 11 GRADE 50 OR GRADE 60	8 BAR DIAMETERS

[A] DOES NOT APPLY TO STIRRUPS OR TIES



10 REBAR SPECIFICATIONS

SCALE: 3/4" = 1'-0"



NOTE: MINIMUM BOLT SPACING SHALL BE 12 x BOLT DIAMETERS WITH A MINIMUM EDGE DISTANCE OF 6 DIAMETERS (CONTRACTOR OPTION TO USE EPOXY GROUTED ANCHORS IN LIEU OF C.I.P. ANCHORS, WHERE AND WHEN APPROVED BY THE ENGINEER OF RECORD)

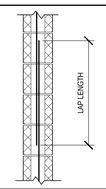
BOLT SIZE	MIN EMBEDMENT
1/2"	4"
5/8"	5"
3/4"	6"
7/8"	6"

11 TYPICAL WALL BOLT EMBEDMENT

SCALE: 1/2" = 1'-0"

LAP SPLICE LENGTHS FOR CMU WALL REINFORCEMENT
(GRADE 60 UNCOATED BARS IN FULLY GROUTED CELLS)

BAR SIZE	LAP LENGTH
# 3	2'-3"
# 4	3'-0"
# 5	3'-9"
# 6	4'-6"
# 7	5'-3"
# 8	6'-0"
# 9	7'-7"



NOTES:
1. LAP SPLICE LENGTHS ARE TO BE INCREASED BY 50% WHEN EPOXY COATED REINFORCEMENT IS USED.
2. BARS LARGER THAN #9 ARE REQUIRED TO BE SPLICED BY MECHANICAL CONNECTORS.

12 CMU LAP SPLICES

SCALE: 3/4" = 1'-0"

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PROJECT NUMBER: 2338.0

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REVISIONS		
#	Description	Date

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CMU DETAILS

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0 1/2" 1"
SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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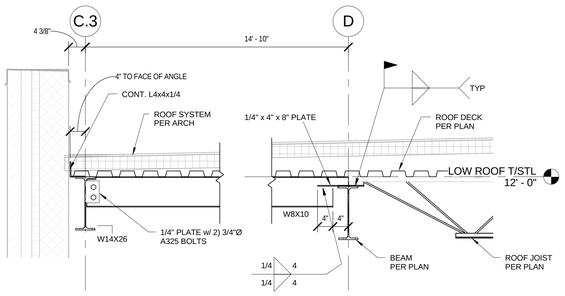
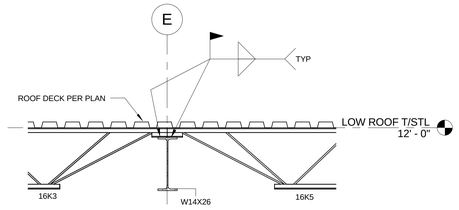


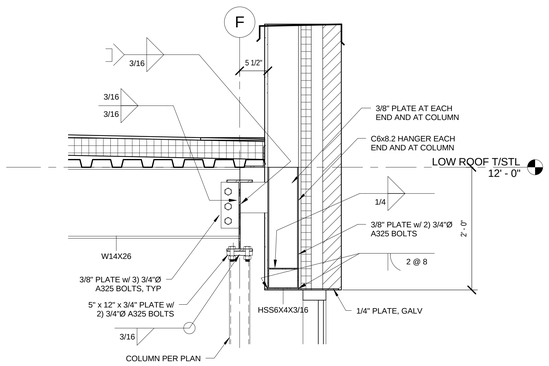
1 BEAM/JOIST-TO-BEAM CONNECTION

H-S2.1 SCALE: 3/4" = 1'-0"



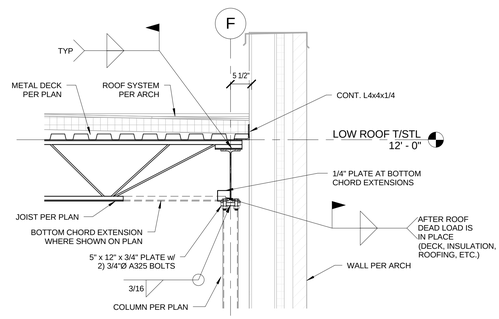
2 JOIST-TO-BEAM CONNECTION

H-S2.1 SCALE: 3/4" = 1'-0"



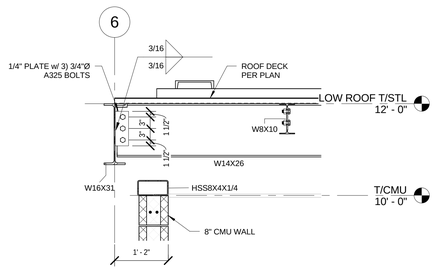
3 FRAMING AT LARGE WINDOW OPENING

H-S2.1 SCALE: 1" = 1'-0"



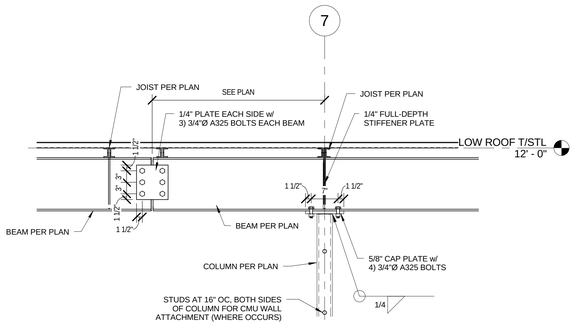
4 JOIST-TO-BEAM AT EXT. WALL

H-S2.1 SCALE: 3/4" = 1'-0"



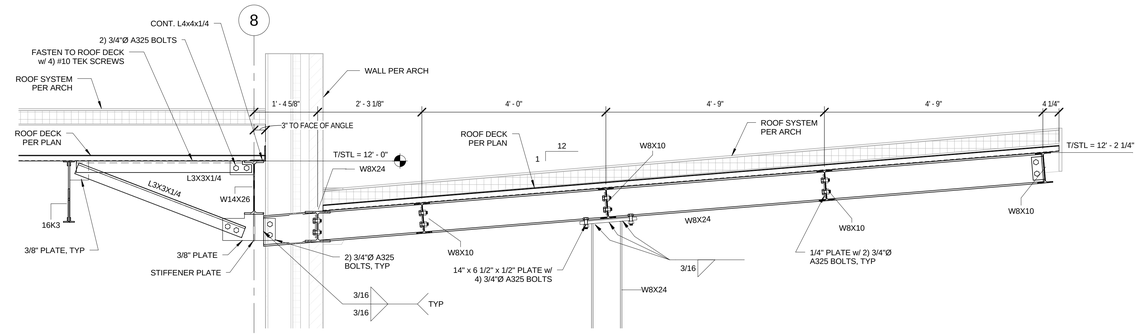
5 LOW ROOF FRAMING TO BEAM

H-S2.1 SCALE: 3/4" = 1'-0"



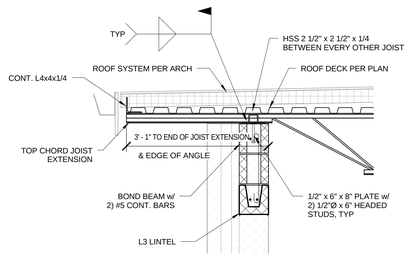
6 BEAM-TO-BEAM CONNECTION

H-S2.1 SCALE: 3/4" = 1'-0"



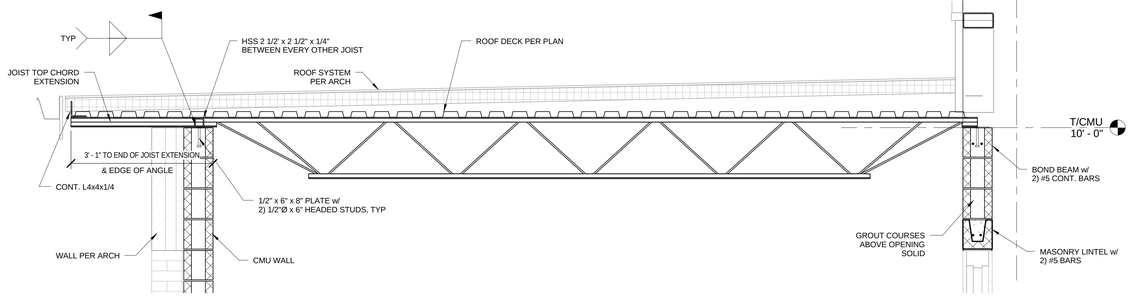
7 CANOPY FRAMING

H-S2.1 SCALE: 3/4" = 1'-0"



8 CMU LINTEL AT STORAGE

H-S2.1 SCALE: 3/4" = 1'-0"



9 STORAGE JOIST-TO-CMU FRAMING

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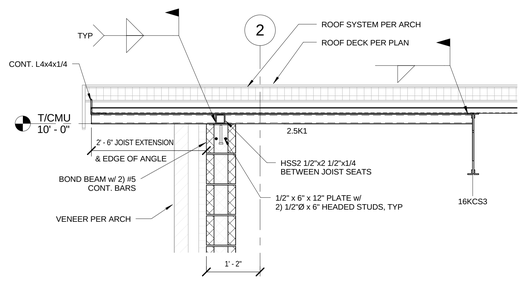
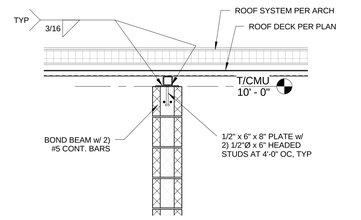


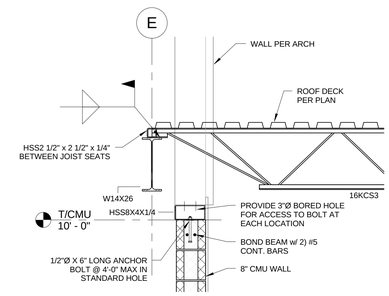
10 CMU BEARING W/ OVERHANG AT STORAGE

H-S3.1 SCALE: 3/4" = 1'-0"



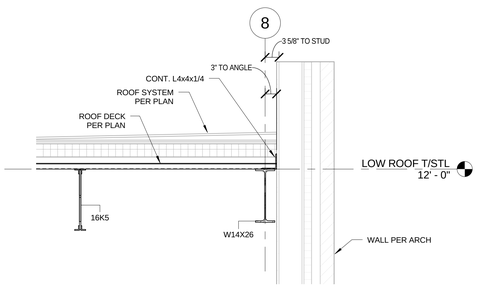
11 CMU BEARING AT STORAGE

H-S3.1 SCALE: 3/4" = 1'-0"



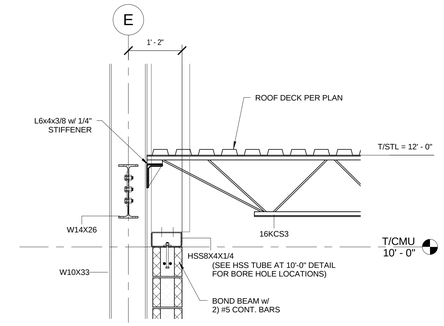
12 JOIST TO BEAM AT 'E'

H-S2.1 SCALE: 3/4" = 1'-0"



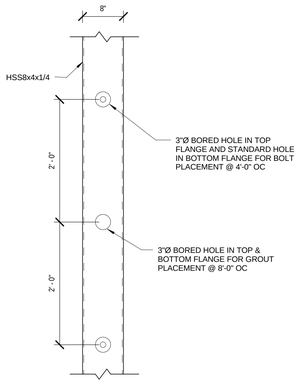
13 BEAM AT PARAPET

H-S2.1 SCALE: 3/4" = 1'-0"



14 JOIST TO COLUMN AT 'E'

H-S2.1 SCALE: 3/4" = 1'-0"



15 HSS TUBE AT 10'-0"

SCALE: 1" = 1'-0"

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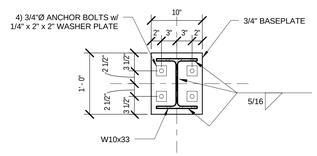
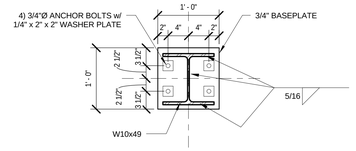


1 BASEPLATE BP-1

SCALE: 1" = 1'-0"



2 BASEPLATE BP-2

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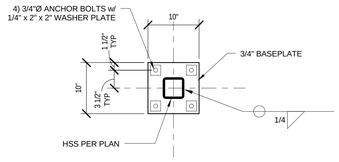
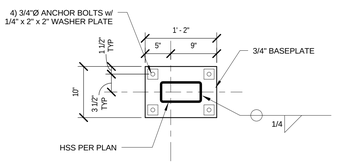


3 BASEPLATE BP-3

SCALE: 1" = 1'-0"



4 BASEPLATE BP-4

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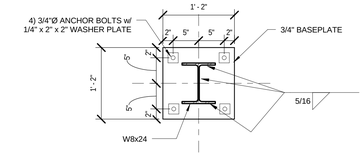
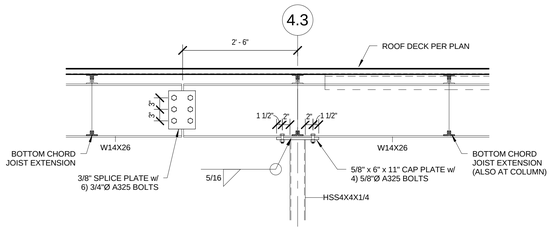


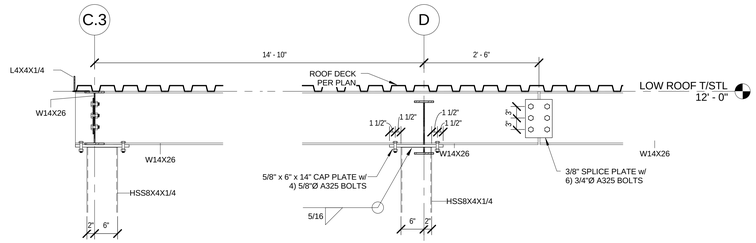
5 BASEPLATE BP-5

SCALE: 1" = 1'-0"



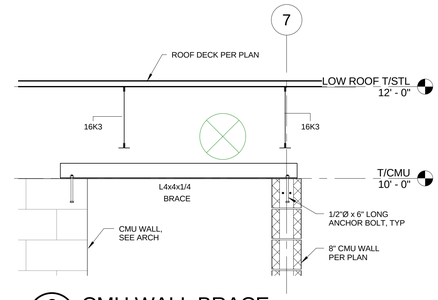
6 HSS 4x4 COLUMN CAP PLATE

H-S2.1 SCALE: 3/4" = 1'-0"



7 HSS 8x4 COLUMN CAP PLATE

H-S2.1 SCALE: 3/4" = 1'-0"



8 CMU WALL BRACE

H-S2.1 SCALE: 3/4" = 1'-0"

9 ROOF OPENING FRAMING

SCALE: 3/4" = 1'-0"

10 MECHANICAL UNIT FRAMING

H-S3.2 SCALE: 3/4" = 1'-0"

11 INTERMEDIATE BRACE

H-S3.2 SCALE: 3/4" = 1'-0"

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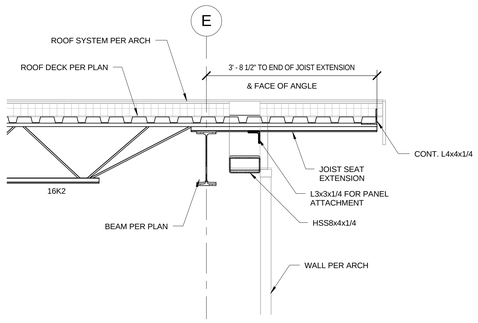
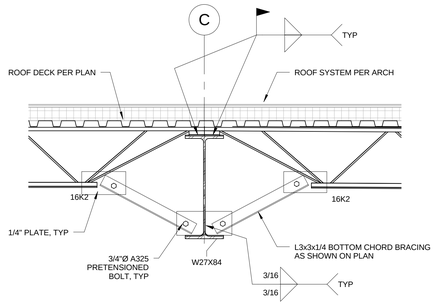


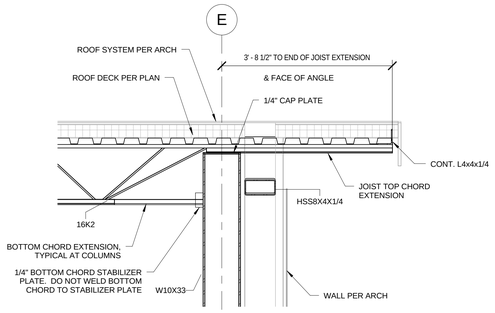
1 ROOF JOIST EXTENSION

H-S2.2 SCALE: 3/4" = 1'-0"



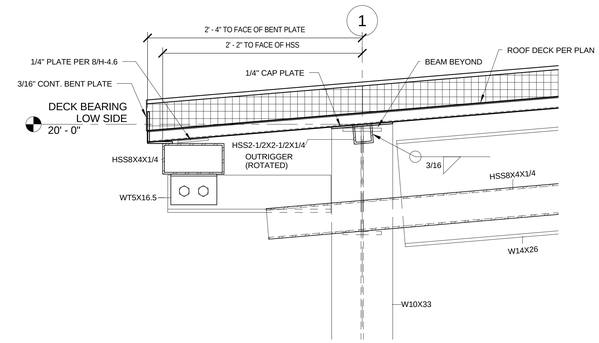
2 ROOF JOIST TO BEAM CONNECTION

H-S2.2 SCALE: 3/4" = 1'-0"



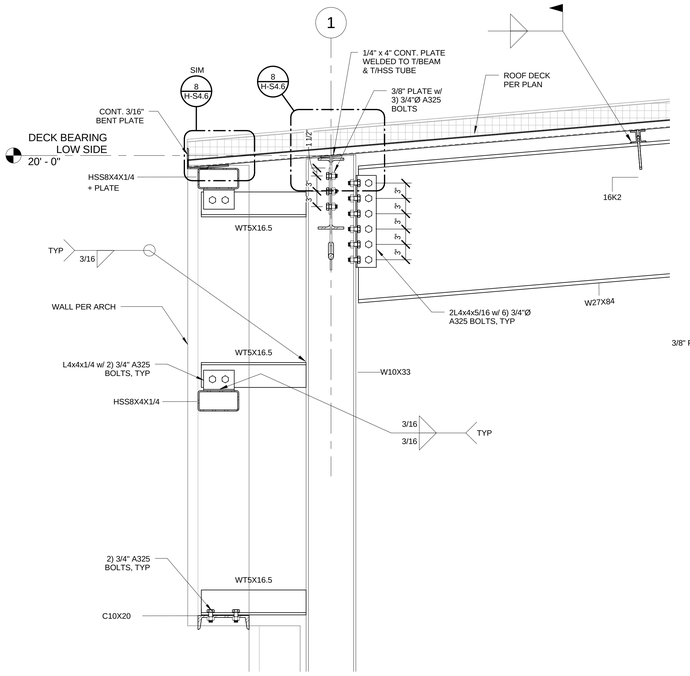
3 ROOF JOIST EXTENSION AT COLUMN

H-S2.2 SCALE: 3/4" = 1'-0"



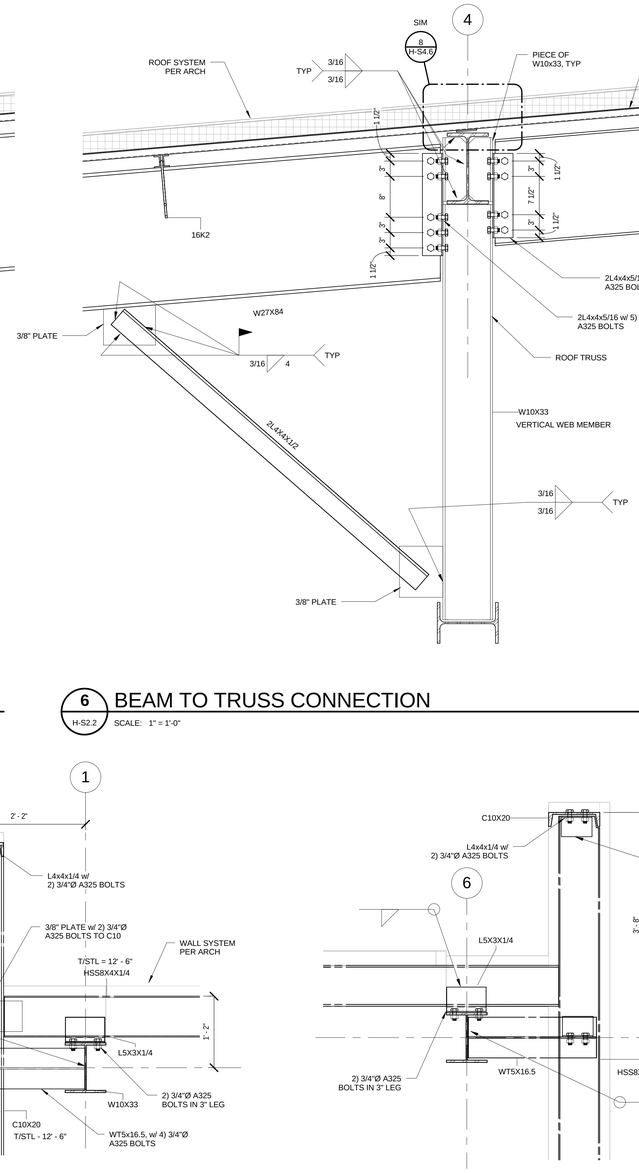
4 OUTRIGGER AT CORNER COLUMN

H-S2.2 SCALE: 1 1/2" = 1'-0"



5 ROOF BEAM TO COLUMN CONNECTION

H-S2.1 SCALE: 1" = 1'-0"



6 BEAM TO TRUSS CONNECTION

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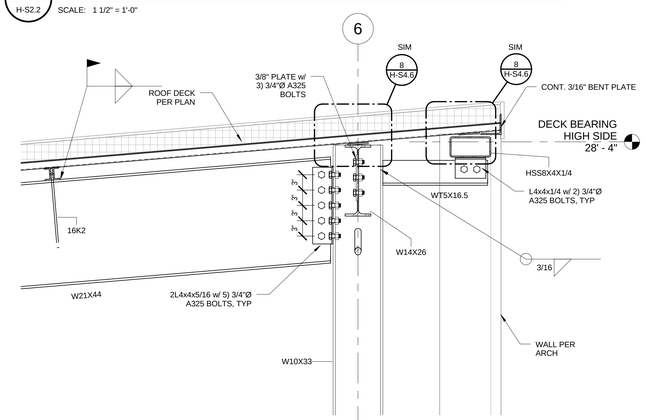
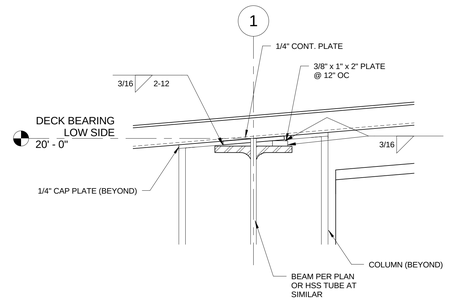


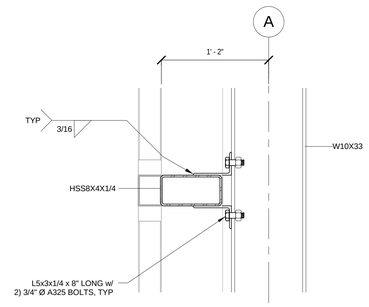
7 ROOF BEAM TO COLUMN

H-S2.2 SCALE: 1" = 1'-0"



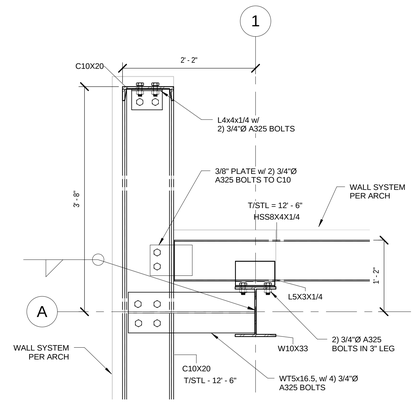
8 PLATE TO BEAM DETAIL

H-S4.6 SCALE: 3" = 1'-0"



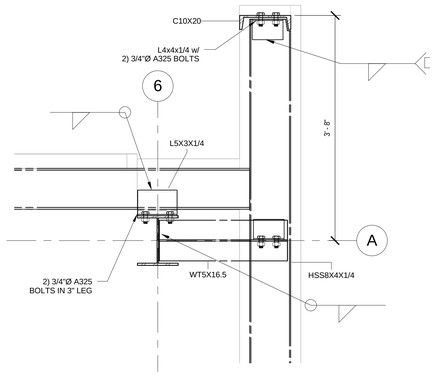
9 GIRT CONNECTION ALONG 'A' & 'E'

H-S3.1 SCALE: 1 1/2" = 1'-0"



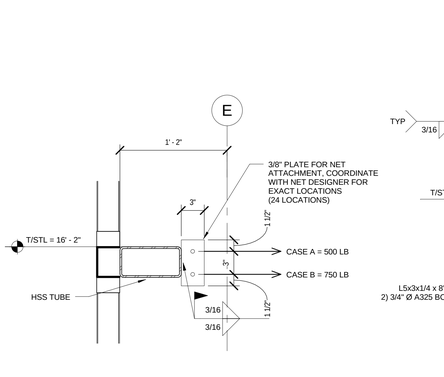
10 CORNER PLAN DETAIL

H-S2.1 SCALE: 1" = 1'-0"



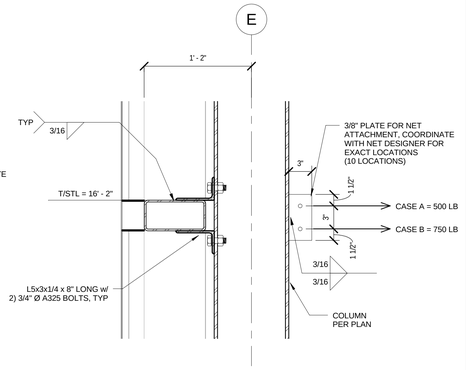
11 HSS CORNER PLAN DETAIL

H-S2.1 SCALE: 1" = 1'-0"



12 NET ATTACHMENT TO HSS

H-S2.3 SCALE: 1 1/2" = 1'-0"



13 NET CONNECTION AT COLUMN

H-S2.2 SCALE: 1 1/2" = 1'-0"

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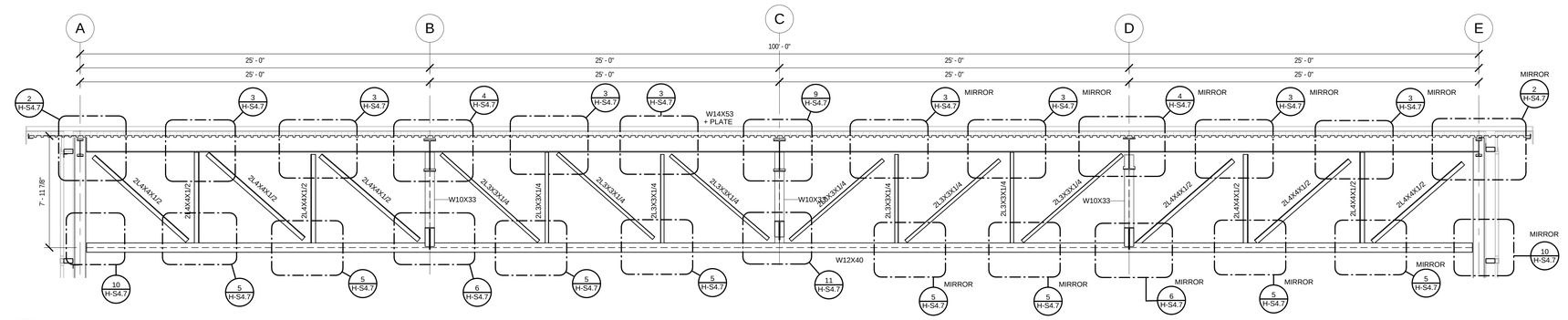


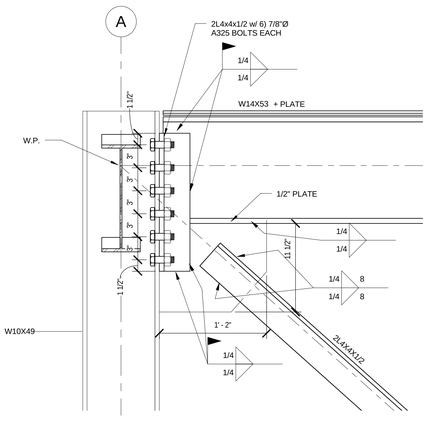
1 ROOF TRUSS T-1

H-S2.2 SCALE: 1/4" = 1'-0"

GENERAL NOTES FOR ROOF TRUSS:

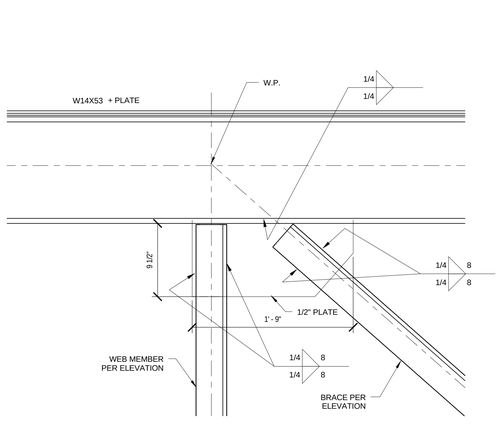
1. TRUSS SHALL BE SHOP WELDED.
2. DOUBLE ANGLE MEMBERS SHALL BE PLACED AS FOLLOWS:
   - A. BACK TO BACK LEGS OF DIAGONALS SHALL TOE DOWN.
   - B. BACK TO BACK LEGS OF VERTICALS SHALL TOE TOWARD CENTER OF TRUSS.
   - C. WEB MEMBERS SHALL HAVE A MINIMUM OF 2 (TWO) STITCH BARS.
3. TRUSS TOP AND BOTTOM CHORDS ARE DESIGNED TO BE CONTINUOUS. DO NOT DETAIL OR FABRICATE SPLICES INTO THE CHORDS WITHOUT APPROVAL OF THE ENGINEER OF RECORD.

   NOTE: TRUSS BOTTOM CHORDS ARE TYPICALLY SIZED TO THEIR MAXIMUM CAPACITY, AND ADDING A BOLTED SPLICE WILL ALMOST CERTAINLY REQUIRE A LARGER BOTTOM CHORD MEMBER SIZE, AT NO ADDITIONAL COST TO THE OWNER
4. HOLES ARE NOT PERMITTED IN THE BOTTOM CHORD.
5. TRUSS SHOP DRAWINGS SHALL SHOW ALL OF THE FOLLOWING:
   - A. INDICATE SHAPE AND GEOMETRY OF TRUSS SHOWING NEUTRAL AXIS OF MEMBERS INTERSECTING AT COMMON WORK POINTS.
   - B. INDICATE THE WELD SIZE AND LENGTH (AT HEEL AND TOE) FOR BOTH ENDS OF ALL WEB MEMBERS.
   - C. INDICATE ALL GUSSET PLATE SIZES AND SIZE OF THE WELD TO CONNECT THEM TO THE TOP AND BOTTOM CHORDS.
   - D. GRADE OF STEEL FOR ALL PIECES.
   - E. TYPE OF WELDING ELECTRODES.



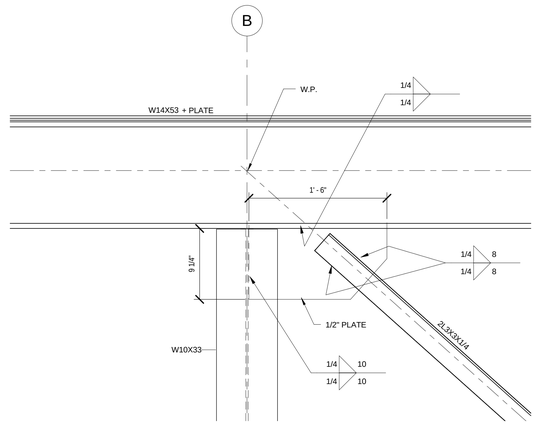
2 ROOF TRUSS END CONNECTION

H-S4.7 SCALE: 1 1/2" = 1'-0"



3 ROOF TRUSS CONNECTION - 2

H-S4.7 SCALE: 1 1/2" = 1'-0"



4 ROOF TRUSS CONNECTION - 3

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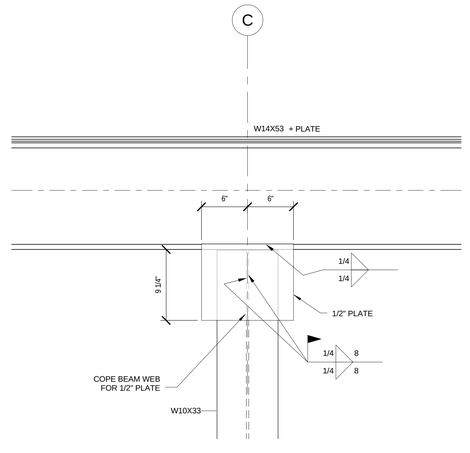

9 ROOF TRUSS CONNECTION - 4

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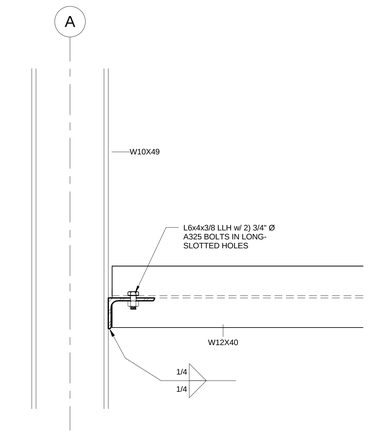
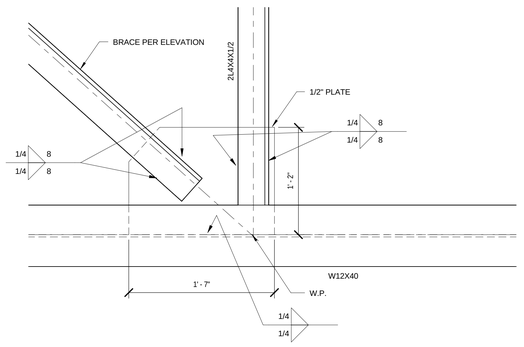


10 ROOF TRUSS CONNECTION - 8

H-S4.7 SCALE: 1 1/2" = 1'-0"



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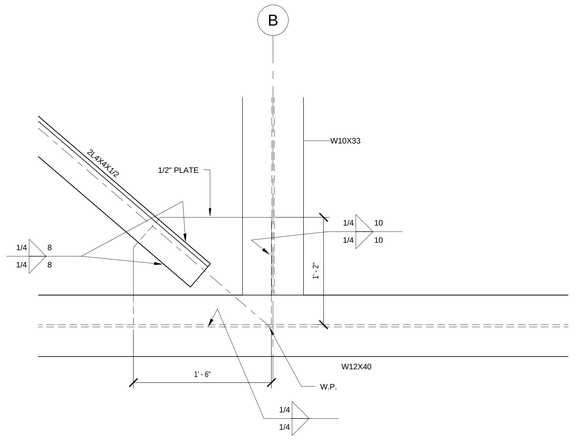
H-S4.7 SCALE: 1 1/2" = 1'-0"



6 ROOF TRUSS CONNECTION - 6

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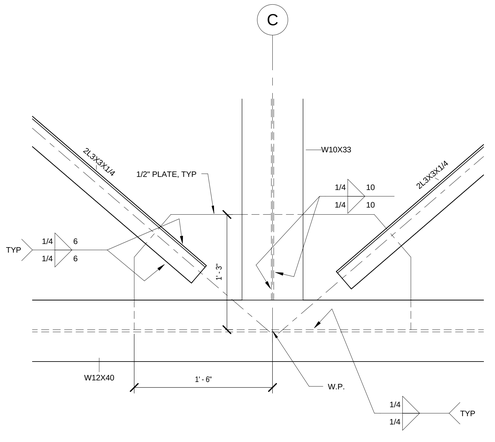

11 ROOF TRUSS CONNECTION - 9

H-S4.7 SCALE: 1 1/2" = 1'-0"

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COMMONWEALTH OF KENTUCKY KYLE S. BESSLER 23151 LICENSED PROFESSIONAL ENGINEER

University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

COMM. NO. 2012.030.00
MADE BY HRG | CHECKED
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HITTING PAVILION
ROOF TRUSS
SECTIONS &
DETAILS
DRAWING NO. **H-S4.7** | BID PKG.

CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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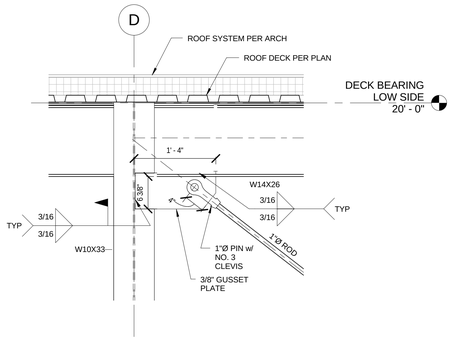

1 X-BRACING AT '1' - CONNECTION 1

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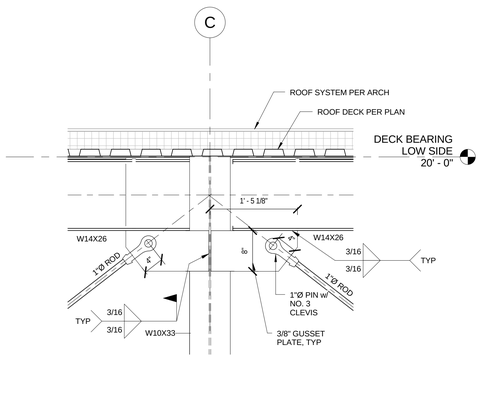

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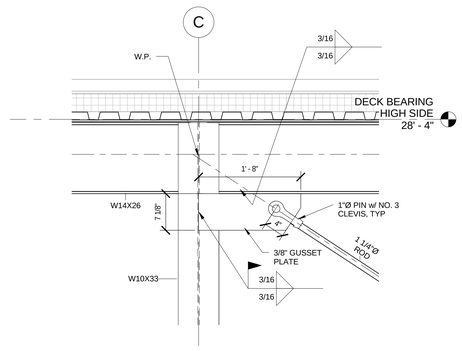

3 X-BRACING AT '6' - CONNECTION 1

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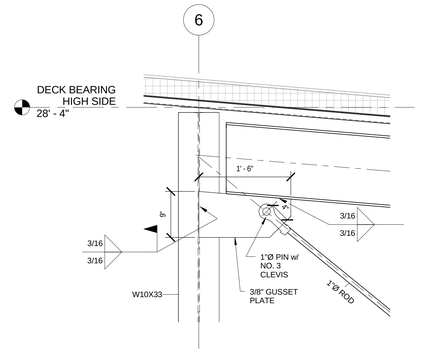

4 X-BRACING AT 'E' - CONNECTION 1

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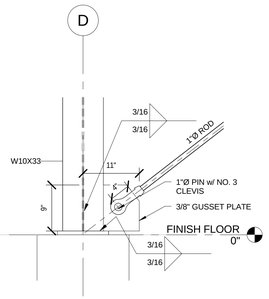

5 X-BRACING AT '1' - CONNECTION 3

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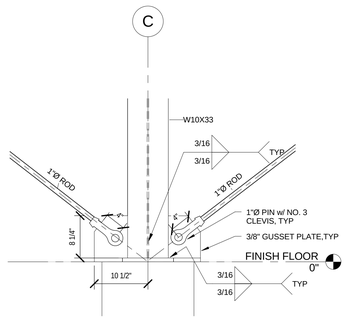

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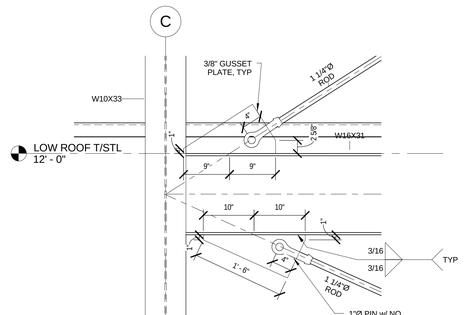

7 X-BRACING AT '6' - CONNECTION 2

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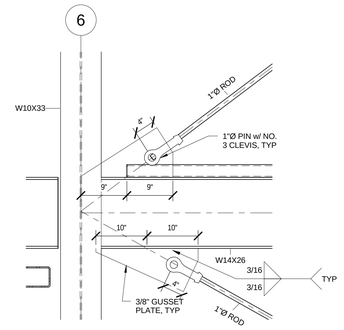

8 X-BRACING AT 'E' - CONNECTION 2

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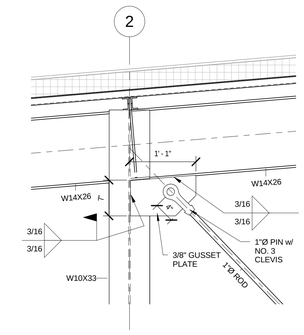

9 X-BRACING AT 'A' - CONNECTION 1

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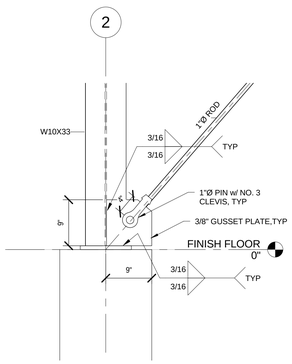

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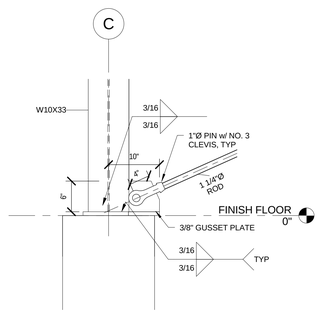
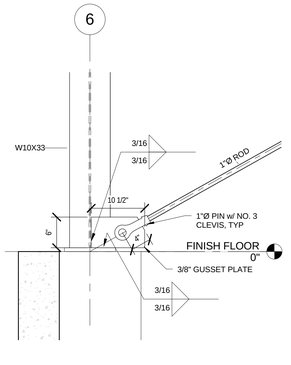


11 X-BRACING AT '6' - CONNECTION 3

H-S3.2 SCALE: 1" = 1'-0"



12 X-BRACING AT 'E' - CONNECTION 3

H-S3.3 SCALE: 1" = 1'-0"

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University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

COMM. NO. 2012.030.00
MADE BY HRG | CHECKED
DATE 5/30/12

HITTING PAVILION
BRACING DETAILS

DRAWING NO. H-S4.8 | BID PKG.

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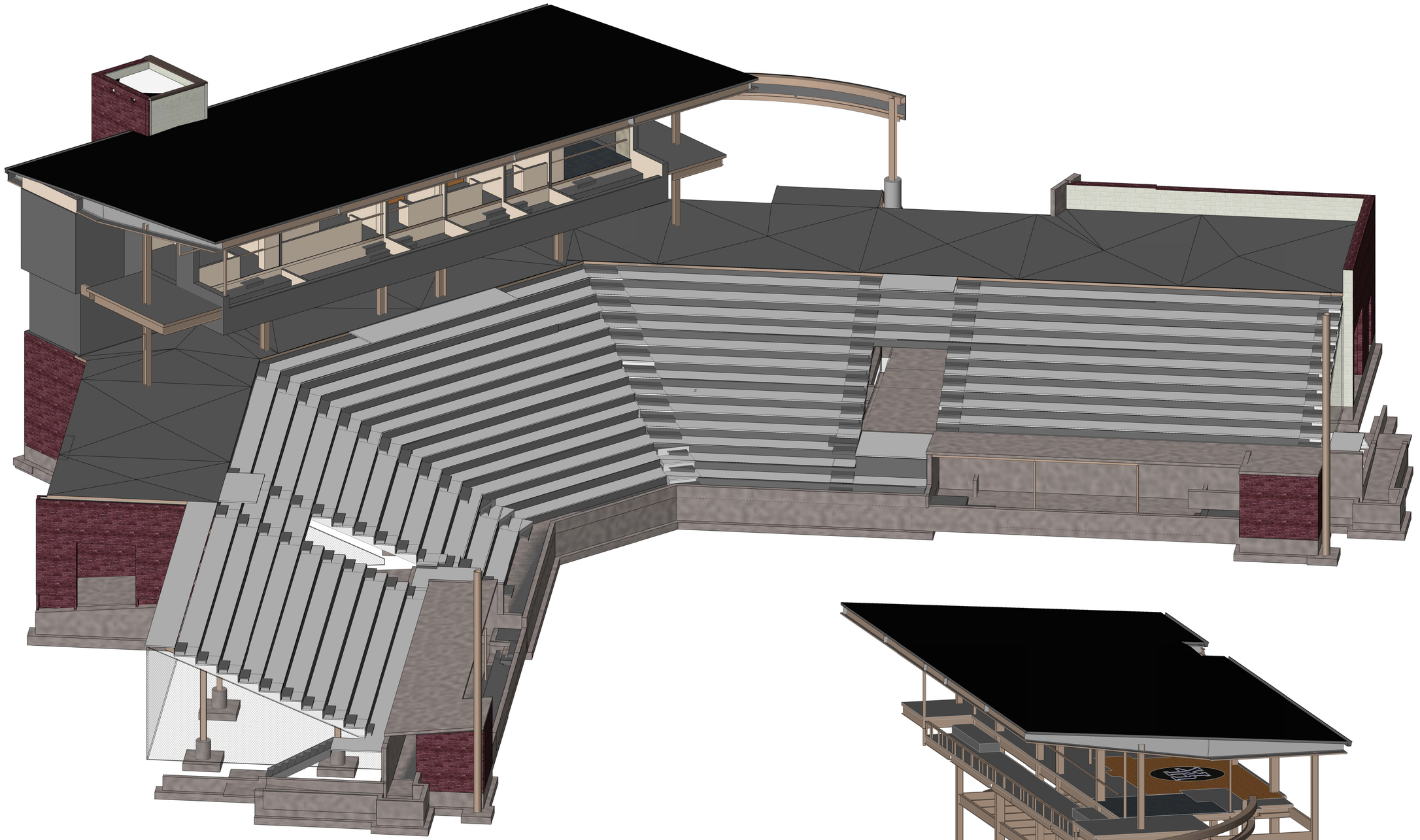
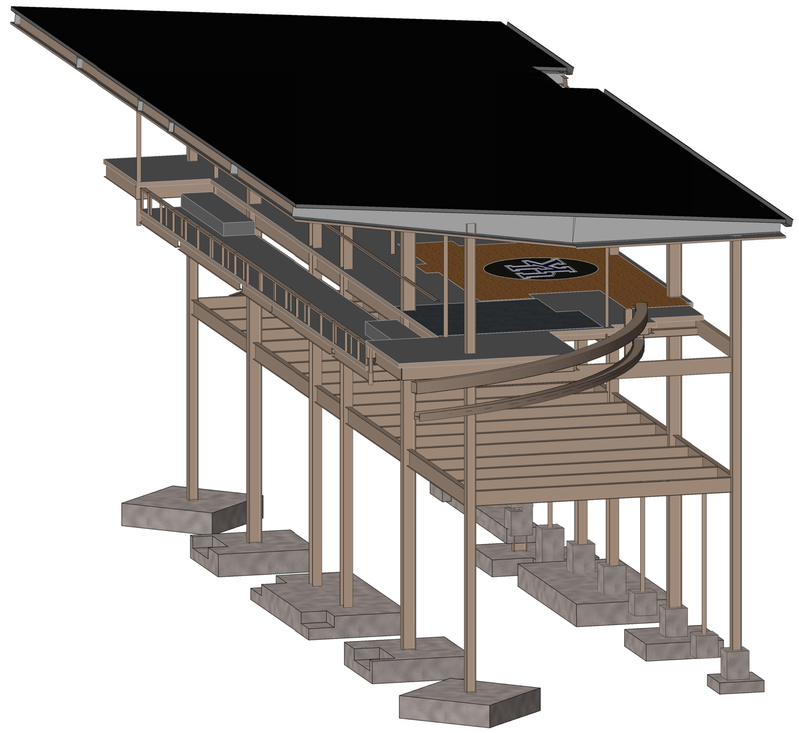


1 STADIUM 3D FRAMING
SCALE:



2 STADIUM PRESS 3D FRAMING
SCALE:

CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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University of Kentucky
Renovate/Upgrade Softball Complex

PROJECT NUMBER: 2338.0

SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

U.K

REVISIONS		
#	Description	Date

COMM. NO. 2012.030.00
MADE BY HRG
CHECKED
DATE 5/30/12

STADIUM 3D FRAMING VIEWS

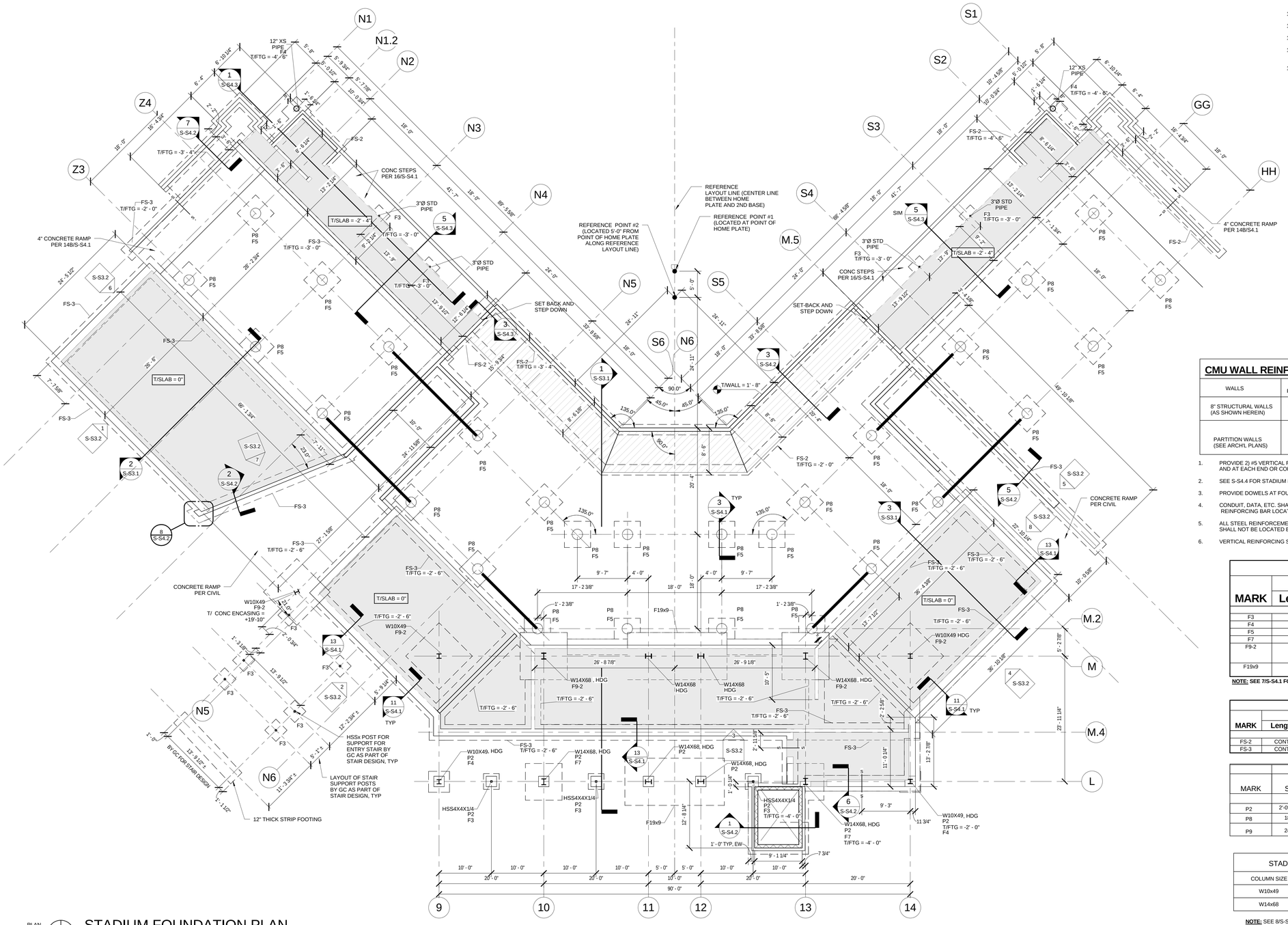
DRAWING NO. S-S1.0
BID PKG.

CONTRACTOR NOTE:

NOTE THAT THE EXISTING UTILITIES INDICATED ON THE SITE SURVEY REFLECT THE MOST CURRENT SITE INFORMATION AVAILABLE, HOWEVER THERE MAY BE VARIOUS EXISTING ACTIVE UTILITIES BELOW GRADE THAT ARE NOT REFLECTED ON THE SURVEY. CARE SHALL BE TAKEN BY THE CONSTRUCTION TEAM DURING EXCAVATION PROCEDURES TO PROTECT ALL EXISTING UTILITIES.

LEGEND

- INDICATES INTERIOR 4" THICK CONCRETE SLAB-ON-GRADE w/ 6x6-W2.9xW2.9 WWR ON VAPOR RETARDER ON 2" SAND BASE ON 4" GRANULAR STONE SUBBASE ON GRADE. SEE 2/S-S4.1. PROVIDE CONTROL JOINTS PER 2 & 10/S4.1
- 6" MIN CONCRETE SLAB w/ #4 BARS @ 16" OC, E.W. ON 4" GRANULAR STONE SUBBASE ON GRADE, SEE 2/S-S4.1.
- INDICATES 8" CMU WALL PER SCHEDULE
- INDICATES CONRETE CURB/WALL
- F.O.W. — INDICATES GRID @ FACE OF WALL
- INDICATES STEP IN FOOTING PER 12/S4.1
- INDICATES BAY WHERE NO BLEACHER BRACING IS PERMITTED
- INDICATES HOT DIPPED GALVANIZED (EXPOSED) COLUMN.



PLAN NORTH

STADIUM FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

FOUNDATION NOTES:

1. THE FOUNDATIONS HAVE BEEN DESIGNED BASED ON RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL INVESTIGATION REPORT PREPARED BY AMEC, DATED MARCH 21, 2012.
2. FOUNDATIONS SHALL BEAR ON PROPERLY PREPARED AND COMPACTED SOILS AS OUTLINED IN THE GEOTECHNICAL REPORT REFERENCED ABOVE.
3. PROTECT EXISTING UTILITIES AND STRUCTURES (OVERHEAD OR UNDERGROUND) WITHIN THE WORK AREA AS WELL AS ANY EXISTING FOUNDATION SYSTEM(S). THE CONTRACTOR IS RESPONSIBLE FOR ANY SHORING OR BRACING OF SUCH COMPONENTS.
4. FOUNDATIONS WERE DESIGNED UTILIZING KBC SECTION 1605 ' ALTERNATIVE BASIC LOAD COMBINATIONS' WITHOUT THE 1/3 INCREASE IN THE ALLOWABLE BEARING PRESSURES DUE TO SHORT-TERM LOADING.
5. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE CONCRETE OR GROUT HAS ATTAINED FULL DESIGN STRENGTH. CONTRACTOR SHALL BRACE OR PROTECT ALL BUILDINGS AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED FULL DESIGN STRENGTH. CONTRACTOR SHALL PROVIDE FOR DESIGN, PERMITS (AS REQ'D) AND INSTALLATION OF SUCH BRACING.
6. FOUNDATIONS SHALL BE PLACED ACCORDING TO THE DEPTHS SHOWN ON THE DRAWINGS. SHOULD SOIL ENCOUNTERED AT THESE DEPTHS NOT BE APPROVED BY THE INSPECTOR OR SOILS ENGINEER, FOUNDATION ELEVATIONS/DIMENSIONS MAY NEED TO BE MODIFIED BY THE ENGINEER. NOTIFY THE ENGINEER OF RECORD IF THIS IS THE CASE.
7. REFERENCE ELEVATION 0'-0" = ABSOLUTE ELEVATION 1023.5' AS SHOWN ON THE CIVIL DRAWINGS.
8. TOP OF PIER = 0'-0" UNLESS NOTED OTHERWISE.
   TOP OF FOOTING = -2'-0" UNLESS NOTED OTHERWISE.
9. SEE ARCHITECTURE DRAWINGS FOR NON-LOAD BEARING METAL STUD PARTITION WALLS. SEE 4/S-S4.2 FOR THICKENED SLAB @ CMU PARTITION WALLS.
10. SEE S-S4.1 & S-S4.2 FOR STADIUM FOUNDATION SECTIONS AND DETAILS.
11. SEE SHEET S-S4.4 FOR STADIUM MASONRY DETAILS. SEE S0.0 FOR LINTEL SCHEDULE.
12. SEE S-S4.8 AND S-S4.9 FOR STADIUM STEEL AND STEEL DECK DETAILS, RESPECTIVELY.
13. CONTINUOUS FOOTINGS SHALL EXTEND A MINIMUM OF 2'-0" BEYOND END OF WALL, TYP UNO.
14. SEE DETAILS 1 AND 2 S/S-4.5 FOR TYPICAL TOP-OF-OF CMU PARTITION WALL BRACING DETAILS.

DESIGN CRITERIA - STADIUM:

DEAD LOAD:
- CONCOURSE
  - 5 1/2" THICK (AVG.) N.W. SLAB ON METAL DECK (+1/4" OVERLOAD) = 65 PSF
  - FRAMING SELF WEIGHT = 5 PSF
  - COLLATERAL = 10 PSF
  - TOTAL = 80 PSF
- PRESS BOX
  - 4 1/2" THICK N.W. SLAB ON METAL DECK (1/4" OVERLOAD) = 48 PSF
  - FRAMING SELF WEIGHT = 4 PSF
  - COLLATERAL = 10 PSF
  - TOTAL = 62 PSF
- ROOF
  - ROOF DECKING = 3 PSF
  - ROOF FRAMING SELF-WEIGHT = 4 PSF
  - INSULATION = 3 PSF
  - COLLATERAL = 10 PSF
  - TOTAL = 20 PSF
- DUGOUT
  - 6 1/2" THICK N.W. SLAB = 75 PSF

ICE DEAD LOAD:
- SUPPORT POLES FOR STADIUM BACKSTOP NETTING HAS BEEN DESIGNED FOR ICE WEIGHT BASED ON A 25 YEAR MEAN OCCURRENCE
  - ICE LOAD ON NET = 7 PSF

LIVE LOAD:
- CONCOURSE FLOOR = 140 PSF
- PRESS FLOOR = 100 PSF
- DUGOUT ROOF = 100 PSF
- ROOF = 20 PSF

SNOW LOAD:
- GROUND SNOW LOAD = 15 PSF
- FLAT ROOF SNOW LOAD = 9.45 PSF

CMU WALL REINFORCEMENT SCHEDULE - STADIUM

WALLS	VERTICAL REINFORCING	LAP LENGTH	BOND BEAMS	JOINTS
8" STRUCTURAL WALLS (AS SHOWN HEREIN)	#5 AT 32" OC TYP	45"	PER ELEVATIONS & DETAILS	LADDER TYPE W1.7 (9GA.) GALVANIZED WELDED WIRE JOINT REINFORCING AT 16" OC
PARTITION WALLS (SEE ARCH'L PLANS)	#4 AT 48" OC	36"	PER DETAILS	LADDER TYPE W1.7 (9GA.) GALVANIZED WELDED WIRE JOINT REINFORCING AT 16" OC

1. PROVIDE 2) #5 VERTICAL REINFORCEMENT AT JAMBS OF EACH OPENING, AT BOTH SIDES OF CONTROL JOINT; AND AT EACH END OR CORNER OF WALL, UNLESS SHOWN OTHERWISE.
2. SEE S-S4.4 FOR STADIUM MASONRY CONSTRUCTION DETAILS.
3. PROVIDE DOWELS AT FOUNDATION TO MATCH VERTICAL REINFORCING.
4. CONDUIT, DATA, ETC. SHALL BE COORDINATED SUCH THAT THEY OCCUR IN UNREINFORCED VERT. CELLS ONLY. REINFORCING BAR LOCATIONS SHALL HAVE PRECEDENCE. DO NOT CUT REINFORCING BARS.
5. ALL STEEL REINFORCEMENT SHALL BE CONTINUOUS. PROVIDE PROPER LAP SPLICES PER 15/S-S4.4 AS REQ'D. LAP SPLICES SHALL NOT BE LOCATED BETWEEN GROUT LIFTS/CONSTRUCTION JOINTS.
6. VERTICAL REINFORCING SHALL BE GROUTED SOLID WITHIN AN 8" CELL MIN, UNLESS NOTED OTHERWISE ON ELEVATIONS.

STADIUM FOOTING SCHEDULE

MARK	FOOTING SIZE Length	Width	Depth	Reinforcing
F3	3' - 0"	3' - 0"	1' - 0"	4) #4 BARS EW BOTTOM
F4	4' - 0"	4' - 0"	1' - 0"	6) #4 BARS EW BOTTOM
F5	5' - 0"	5' - 0"	1' - 0"	5) #5 BARS EW BOTTOM
F7	7' - 0"	7' - 0"	1' - 6"	7) #5 EW T&B
F9-2	9' - 0"	9' - 0"	2' - 0"	7) #5 BARS EW, BOTTOM 8) #5 BARS EW, TOP 9) #8 BARS E.W. BOTTOM
F19x9	9' - 0"	19' - 0"	2' - 0"	7) #5 BARS SHORT & 16#5 TOP BARS @ 14" OC E/W, #8 BOT BARS @ 12" OC E/W

NOTE: SEE 7/S-S4.1 FOR TYPICAL FOOTING DETAIL

STADIUM STRIP FOOTING SCHEDULE

MARK	FOOTING SIZE Length	Width	Thickness	REINFORCING Longitudinal	Transverse
FS-2	CONT.	2' - 0"	1' - 0"	2) #5	#5 @ 16" OC
FS-3	CONT.	3' - 0"	1' - 4"	3) #5	#5 @ 8" OC

STADIUM PIER SCHEDULE

MARK	SIZE	REINFORCING VERTICAL	REINFORCING TIES	T/PIER ELEV.	DETAIL
P2	2'-0" x 2'-0"	8) #6	#4 @ 8" OC	+6"	3/S-S4.1
P8	18" Ø	NA	#3 SPIRAL 8" OC	+6", TYP UNO	4/S-S4.1
P9	24" Ø	6) #6	#4 SPIRAL 8" OC	+6"	3/S-S4.1

STADIUM COLUMN / BASEPLATE / ANCHOR BOLT SCHEDULE

COLUMN SIZE	BASEPLATE (t x L x W)	ANCHOR BOLT SIZE	EMBEDMENT
W10x49	3/4" x 12" x 12"	BP-2	1" Ø	9"
W14x68	1" x 16" x 12"	BP-4	1 1/8" Ø	16"

NOTE: SEE 8/S-S4.8 FOR BASE PLATE CONFIGURATION.

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KYLE S. BESSLER 29151 LICENSED PROFESSIONAL ENGINEER STATE OF KENTUCKY

University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR / COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

COMM. NO. 2012.030.00
MADE BY HRG | CHECKED
DATE 5/30/12
STADIUM FOUNDATION PLAN
DRAWING NO. S-S1.1 | BID PKG.

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LEGEND

- INDICATES DRAG COLLECTOR ELEMENT. PROVIDE CLASS "A" FAYING SURFACE W/ A325 BOLTS AND PL 3/8" SHEAR TABS MIN (SEE 1/S-S4.8)
- INDICATES STRUCTURAL WALL BELOW.
- INDICATES NON-STRUCTURAL WALL BELOW.
- [#] INDICATES # OF 3/4" x 3 1/2" STUDS TO BE INSTALLED EVENLY SPACED ALONG THE LENGTH OF BEAM PER 1 & 2/S-S4.9
- INDICATES NON-FRAME MOMENT CONNECTION PER 3/S-S4.8, TYP UNO
- INDICATES MOMENT FRAME CONNECTION PER FRAME ELEV.

FRAMING NOTES:

1. REFER TO S0.0 FOR STRUCTURAL GENERAL NOTES.
2. S1 - DENOTES 4 1/2" TOTAL DEPTH NORMAL WEIGHT CONCRETE SLAB ON 1.3" x 20 GA VENTED DECK (VULCRAFT 'CSV' OR EQUAL). REINFORCE SLAB WITH #4 @ 12" OC NESTED WITHIN LOW FLUTE AND 6x6-W2.9xW2.9 WWR ADEQUATELY BOLSTERED/CHAIRED. **SEE 4/S-S4.9 FOR TYPICAL CROSS-SECTION OF THIS CONDITION.**
   SLOPE SLAB AS PER ARCH'L TO MAX OF 6 1/2", MIN TOTAL DEPTH.
   FASTENING: SUPPORT: 5/8" PUDDLE WELDS (32/4 PATTERN)
   SIDELAPS: #10 TEK SCREWS @ 12" OC
3. S2 - DENOTES 5" TOTAL DEPTH NORMAL WEIGHT CONCRETE SLAB ON 2" x 20 GA DECK. REINFORCE SLAB WITH 6x6-W2.9xW2.9 WWR ADEQUATELY BOLSTERED/CHAIRED.
   FASTENING: SUPPORT: 5/8" PUDDLE WELDS @ 12" OC AVG (36/4 PATTERN)
   SIDELAPS: #10 TEK SCREWS @ 12" OC
4. R1 - DENOTES 1 1/2" x 22 GA WIDE RIB (B) ROOF DECK.
   FASTENING: SUPPORT: 5/8" PUDDLE WELDS @ 12" OC
   SIDELAPS: #10 TEK SCREWS @ 12" OC
5. SEE ARCH FOR NON-LOAD BEARING METAL STUD AND CMU PARTITION WALLS.
6. AT ALL ROOF OPENINGS/PENETRATIONS PROVIDE AS PER 5, 6, 7/S-S4.9 ACCORDINGLY.
7. SEE 1, 2/S-S4.5 FOR TYPICAL TOP OF CMU PARTITION WALL BRACING DETAILS.
8. SPACE BEAMS EQUALLY BETWEEN COLUMN GRIDLINES UNLESS NOTED OTHERWISE.
9. **ALL EXPOSED COLUMNS AND BEAMS SHALL BE HOT DIPPED GALVANIZED (HDG). CORRESPONDINGLY, ALL BOLTS, WASHERS SHALL BE HDG. ALL EXPOSED WELDS SHALL BE PAINTED WITH GALVANIZING COATING PRIOR TO BEING PAINTED.**

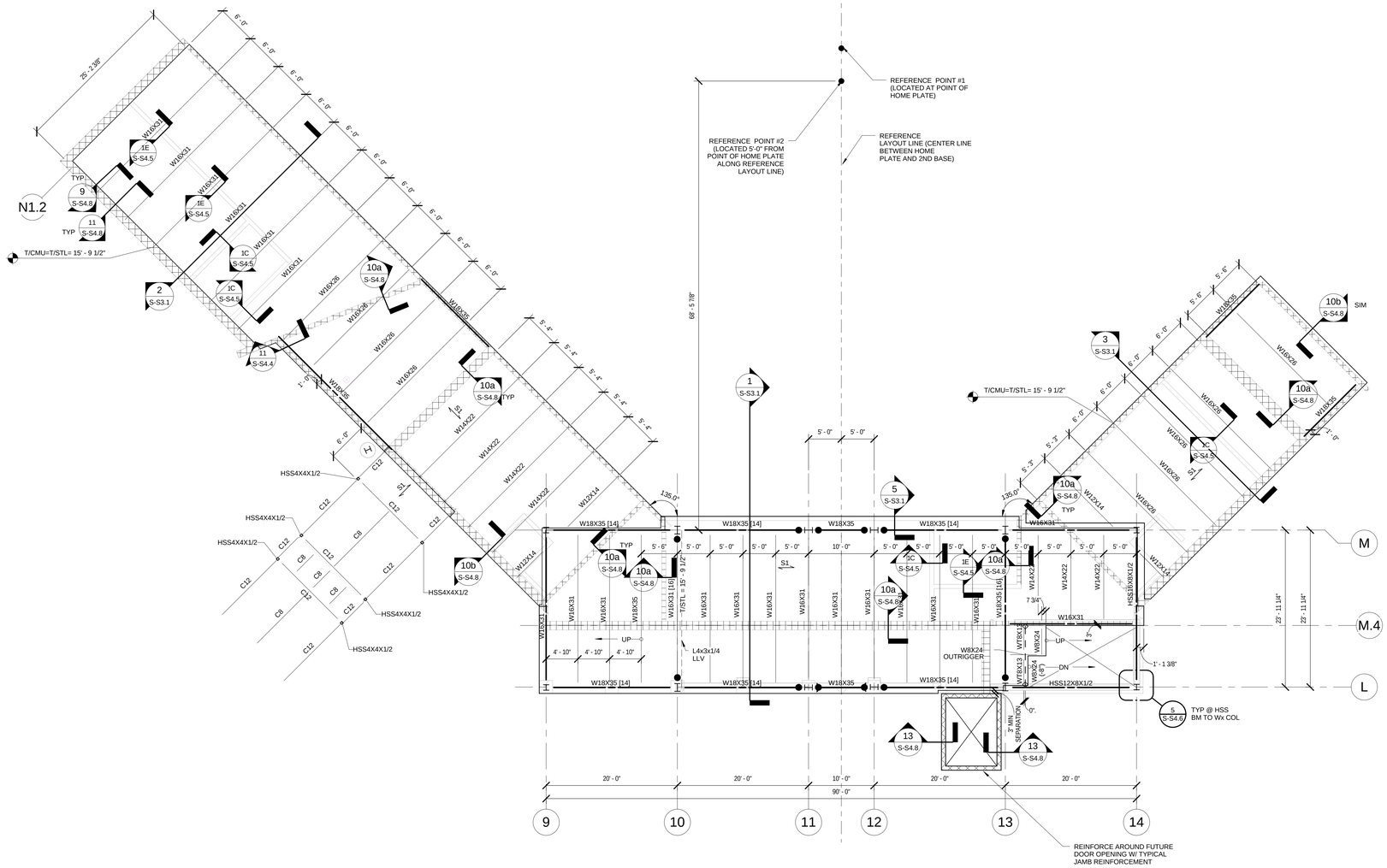
AISC STD. SHAPE & QTY OF HEADED STUDS PER 1&2/S-S4.9 — W14x22 [20] 1" — CAMBER
(2 1/2") — T/ STEEL RELATIVE TO TYPICAL T/ STEEL

DESIGN CRITERIA - STADIUM:

DEAD LOAD:
CONCOURSE
5 1/2" THICK (AVG.) N.W. SLAB ON METAL DECK (+1/4" OVERLOAD) = 65 PSF
FRAMING SELF WEIGHT = 5 PSF
COLLATERAL = 10 PSF
TOTAL = 80 PSF

LIVE LOAD:
CONCOURSE FLOOR = 140 PSF

SNOW LOAD:
GROUND SNOW LOAD = 15 PSF
FLAT ROOF SNOW LOAD = 9.45 PSF



PLAN NORTH

STADIUM CONCOURSE FRAMING PLAN

SCALE: 1/8" = 1'-0"

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DRAWING NO. S-S2.1 | BID PKG.

CONSTRUCTION DOCUMENTS

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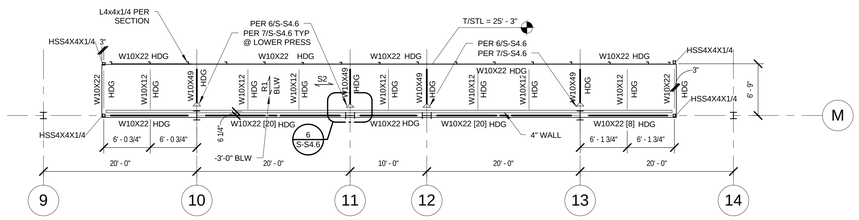
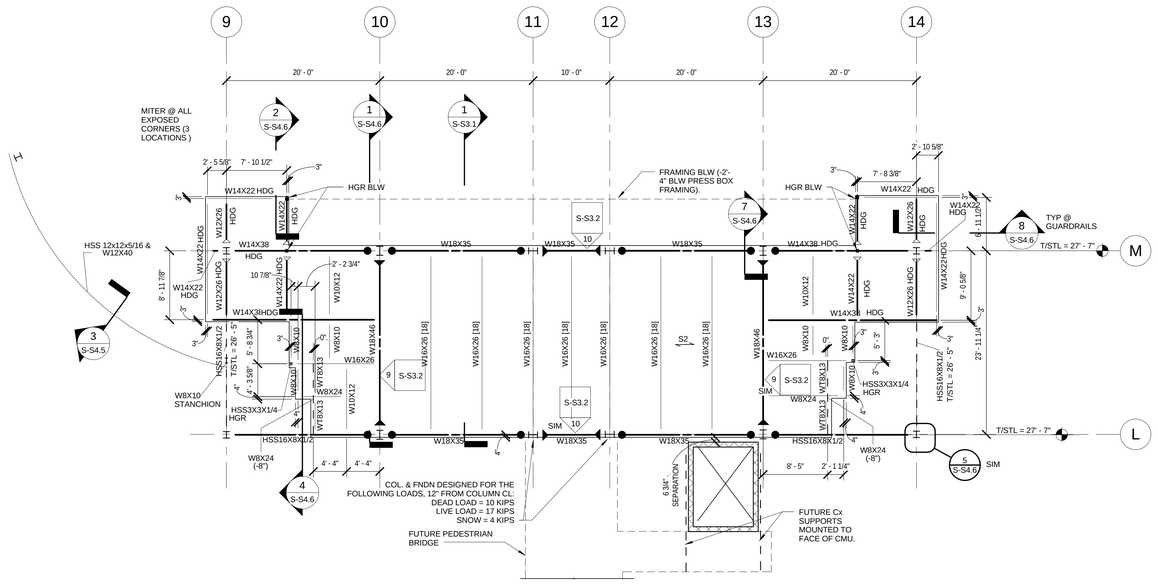


PLAN NORTH

LOWER STADIUM PRESS FRAMING PLAN

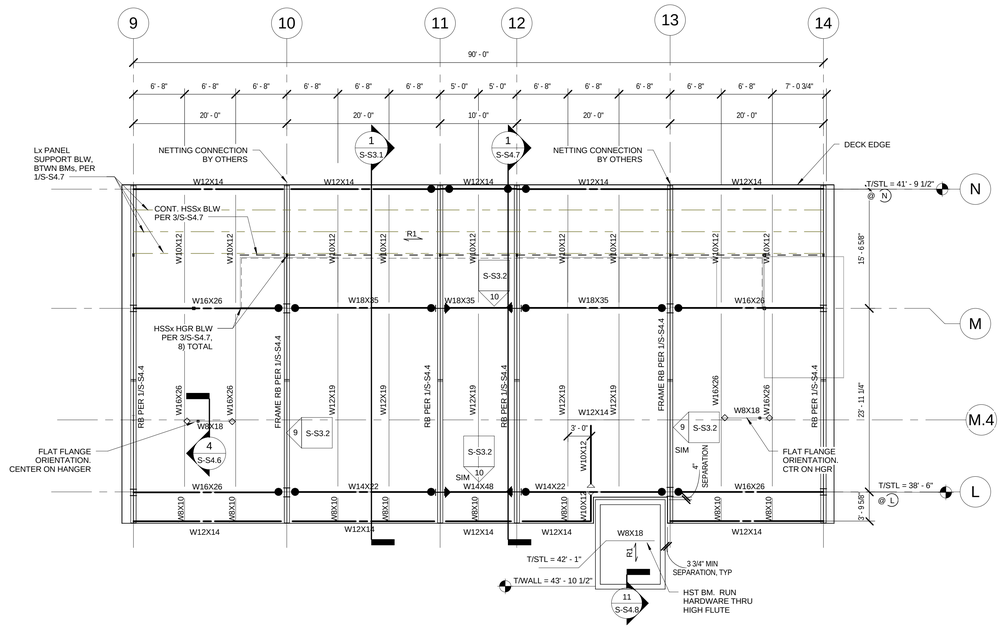
SCALE: 1/8" = 1'-0"



PLAN NORTH

STADIUM PRESS FRAMING PLAN

SCALE: 1/8" = 1'-0"



PLAN NORTH

STADIUM ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

FRAMING NOTES:

1. REFER TO S0.0 FOR STRUCTURAL GENERAL NOTES.
2. S1 - DENOTES 4 1/2" TOTAL DEPTH NORMAL WEIGHT CONCRETE SLAB ON 1.3" x 20 GA VENTED DECK (VULCRAFT 'CSV' OR EQUAL). REINFORCE SLAB WITH #4 @ 12" OC NESTED WITHIN LOW FLUTE AND 6x6-W2.9xW2.9 WWR ADEQUATELY BOLSTERED/CHAIRED. **SEE 4/S-S4.9 FOR TYPICAL CROSS-SECTION OF THIS CONDITION.**

   SLOPE SLAB AS PER ARCH'L TO MAX OF 6 1/2", MIN TOTAL DEPTH.

   FASTENING: SUPPORT: 5/8" PUDDLE WELDS (32/4 PATTERN)
   SIDELAPS: #10 TEK SCREWS @ 12" OC
3. S2 - DENOTES 5" TOTAL DEPTH NORMAL WEIGHT CONCRETE SLAB ON 2" x 20 GA DECK. REINFORCE SLAB WITH 6x6-W2.9xW2.9 WWR ADEQUATELY BOLSTERED/CHAIRED.

   FASTENING: SUPPORT: 5/8" PUDDLE WELDS @ 12" OC AVG (36/4 PATTERN)
   SIDELAPS: #10 TEK SCREWS @ 12" OC
4. R1 - DENOTES 1 1/2" x 22 GA WIDE RIB (B) ROOF DECK.

   FASTENING: SUPPORT: 5/8" PUDDLE WELDS @ 12" OC
   SIDELAPS: #10 TEK SCREWS @ 12" OC
5. SEE ARCH FOR NON-LOAD BEARING METAL STUD AND CMU PARTITION WALLS.
6. AT ALL ROOF OPENINGS/PENETRATIONS PROVIDE AS PER 5, 6, 7/S-S4.9 ACCORDINGLY.
7. SEE 1, 2/S-S4.5 FOR TYPICAL TOP OF CMU PARTITION WALL BRACING DETAILS.
8. SPACE BEAMS EQUALLY BETWEEN COLUMN GRIDLINES UNLESS NOTED OTHERWISE.
9. **ALL EXPOSED COLUMNS AND BEAMS SHALL BE HOT DIPPED GALVANIZED (HDG), CORRESPONDINGLY, ALL BOLTS, WASHERS SHALL BE HDG. ALL EXPOSED WELDS SHALL BE PAINTED WITH GALVANIZING COATING PRIOR TO BEING PAINTED.**

DESIGN CRITERIA - STADIUM:

DEAD LOAD:

PRESS BOX	
4 1/2" THICK N.W. SLAB ON METAL DECK (1/4" OVERLOAD)	= 48 PSF
FRAMING SELF WEIGHT	= 4 PSF
COLLATERAL	= 10 PSF
TOTAL	= 62 PSF
ROOF	
ROOF DECKING	= 3 PSF
ROOF FRAMING SELF-WEIGHT	= 4 PSF
INSULATION	= 3 PSF
COLLATERAL	= 10 PSF
TOTAL	= 20 PSF

ICE DEAD LOAD:
SUPPORT POLES FOR STADIUM BACKSTOP NETTING HAS BEEN DESIGNED FOR ICE WEIGHT BASED ON A 25 YEAR MEAN OCCURRENCE
ICE LOAD ON NET = 7 PSF

LIVE LOAD:

PRESS FLOOR	= 100 PSF
ROOF	= 20 PSF

SNOW LOAD:

GROUND SNOW LOAD	= 15 PSF
FLAT ROOF SNOW LOAD	= 9.45 PSF

LEGEND

- INDICATES DRAG COLLECTOR ELEMENT. PROVIDE CLASS "A" FAYING SURFACE W/ A325 BOLTS AND PL 3/8" SHEAR TABS MIN (SEE 1/S-S4.8)
- INDICATES STRUCTURAL WALL BELOW.
- [#] INDICATES # OF 3/4" x 3 1/2" STUDS TO BE INSTALLED EVENLY SPACED ALONG THE LENGTH OF BEAM PER 1 & 2/S-S4.9
- INDICATES NON-FRAME MOMENT CONNECTION PER 3/S-S4.8, TYP UNO
- INDICATES MOMENT FRAME CONNECTION PER FRAME ELEV.

CONSTRUCTION DOCUMENTS

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MADE BY	Author
CHECKED	Checker
DATE	5/30/12

STADIUM PRESS BOX AND ROOF FRAMING PLANS

DRAWING NO. **S-S2.2** BID PKG.

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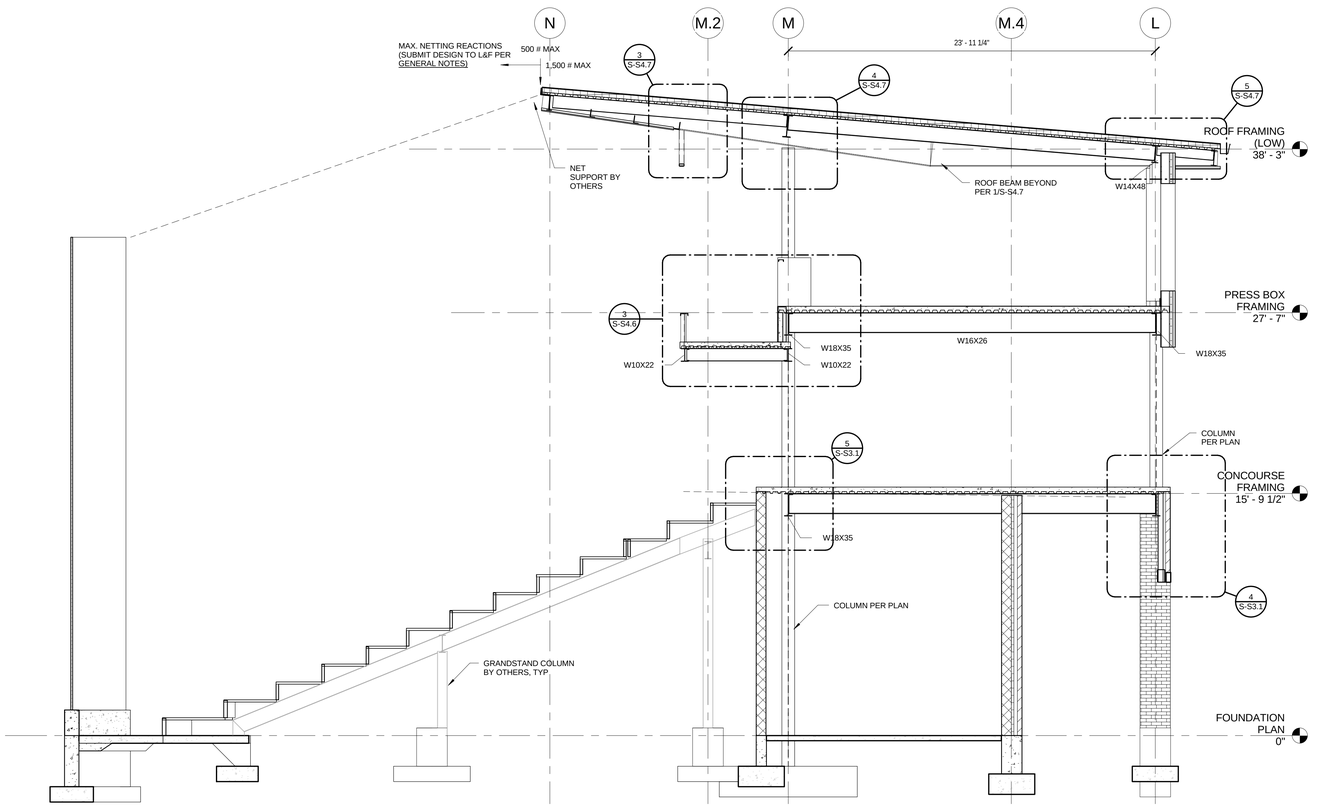
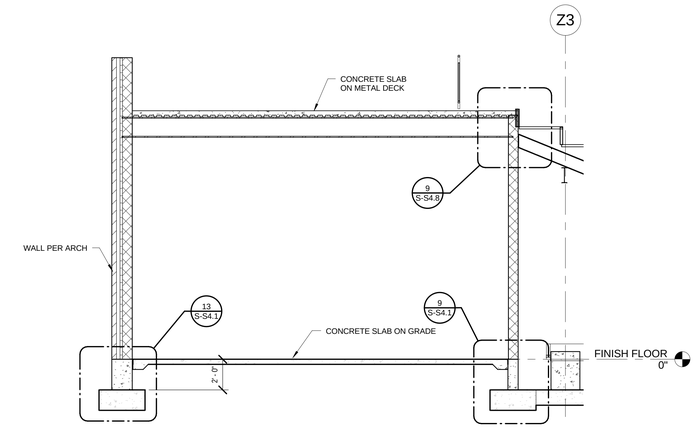
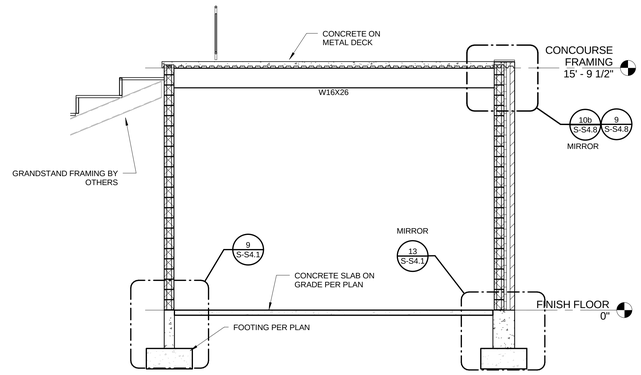


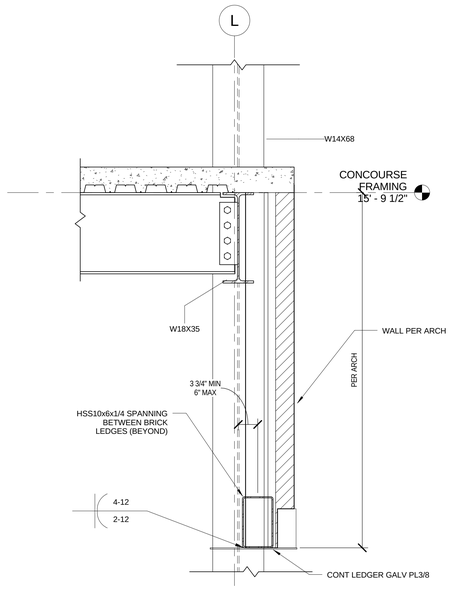
1 STADIUM BUILDING SECTION THRU PRESS
S-S1.1 SCALE: 1/4" = 1'-0"



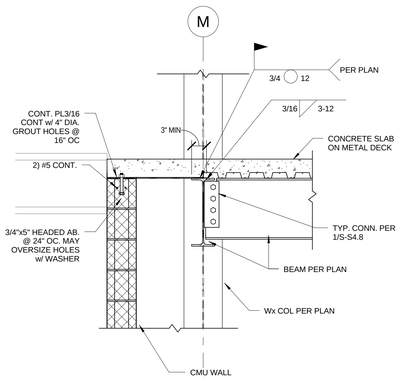
2 NORTH CONCOURSE SECTION
S-S1.1 SCALE: 1/4" = 1'-0"



3 SOUTH CONCOURSE SECTION
S-S1.1 SCALE: 1/4" = 1'-0"



4 CONCOURSE VENEER SUPPORT
S-S3.1 SCALE: 1" = 1'-0"



5 CONCOURSE TO CMU CONNECTION
S-S2.1 SCALE: 3/4" = 1'-0"

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	STADIUM BUILDING SECTIONS
DRAWING NO.	S-S3.1
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CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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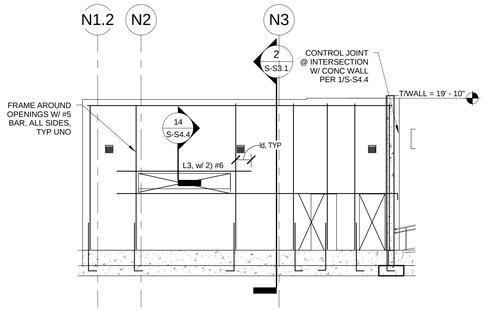
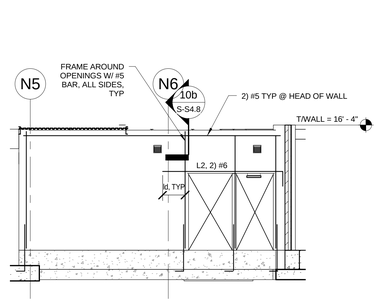


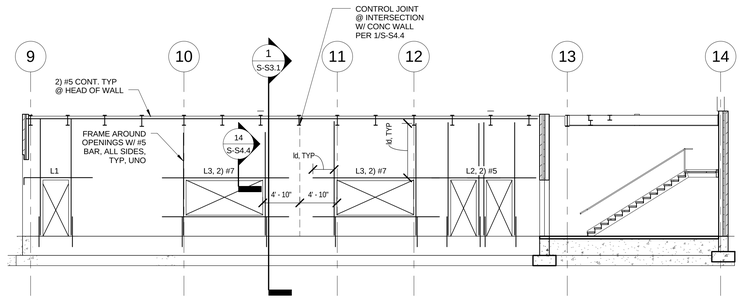
1 ELEV AT GRID Z1

S-S1.1 SCALE: 1/8" = 1'-0"



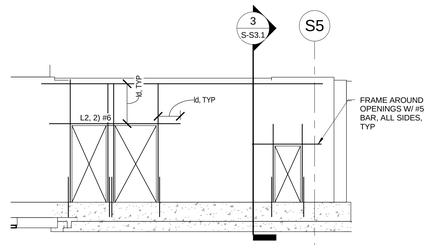
2 ELEV AT GRID Z1.2

S-S1.1 SCALE: 1/8" = 1'-0"



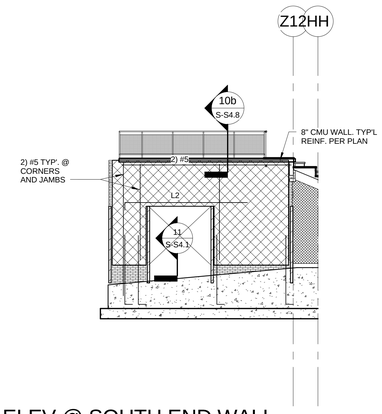
3 ELEV AT GRID L.3

S-S1.1 SCALE: 1/8" = 1'-0"



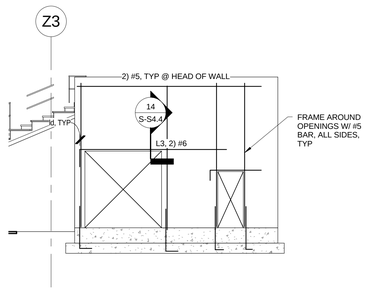
4 ELEV AT GRID Z11

S-S1.1 SCALE: 1/8" = 1'-0"



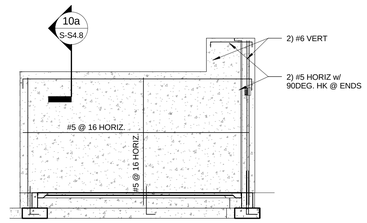
5 ELEV @ SOUTH END WALL

S-S1.1 SCALE: 1/8" = 1'-0"



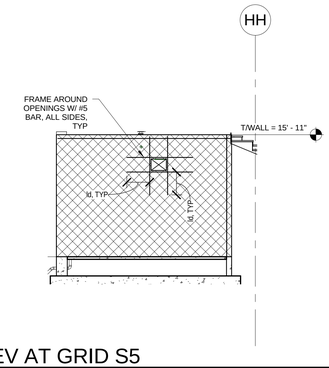
6 ELEV AT GRID N1

S-S1.1 SCALE: 1/8" = 1'-0"



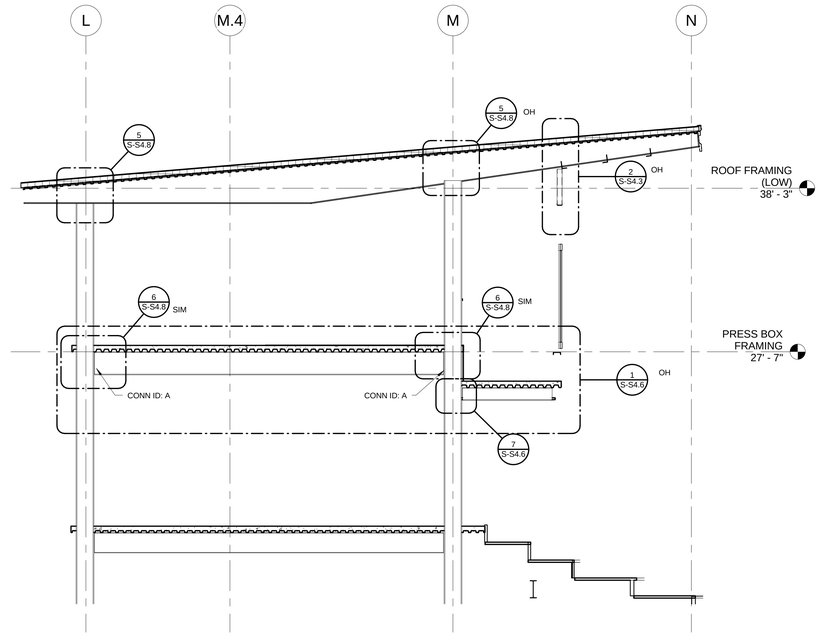
7 ELEV AT GRID N20

S-S1.1 SCALE: 1/8" = 1'-0"



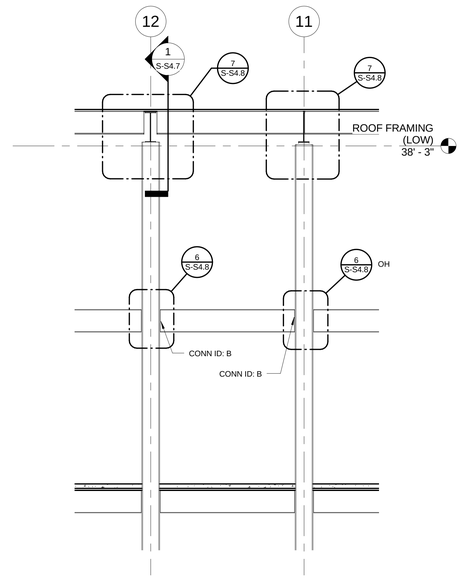
8 ELEV AT GRID S5

S-S1.1 SCALE: 1/8" = 1'-0"



9 LONG BAY FRAME ELEV

S-S2.2 SCALE: 1/4" = 1'-0"



10 SHORT BAY FRAME ELEV

S-S2.2 SCALE: 1/4" = 1'-0"

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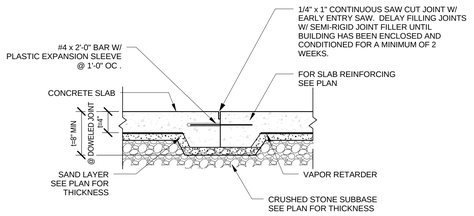
STADIUM WALL & FRAME ELEVATIONS

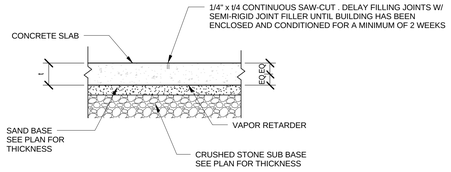
DRAWING NO. **S-S3.2** BID PKG.



NOTE: CONTRACTOR TO VERIFY THE COMPOSITION OF THE SOIL SUBSTRATE AND ITS PREPARATION WITH THE GEOTECHNICAL REPORT.

1 TYP. SLAB ON GRADE CONSTRUCTION JOINT

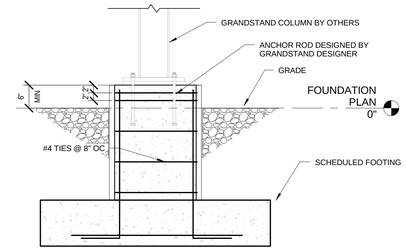
SCALE: 3/4" = 1'-0"



NOTES:
1. SAW CUT JOINTS WITH EARLY ENTRY SAW SUCH AS SOFF-CUT.
2. SAW CUT MUST BE MADE AS SOON AS THE SLAB CAN SUPPORT THE WEIGHT OF THE EQUIPMENT AND CUTTING DOES NOT EXCESSIVELY "TEAR" THE SLAB, BUT NO MORE THAN 12 HOURS AFTER FINISHING.
3. UTILIZE AN ADEQUATE NUMBER OF SAWS TO COMPLETE THE CUTS WITHIN THE REQUIRED TIME.
4. JOINTS MUST BE CUT IN AN APPROXIMATELY SQUARE PATTERN WITH A MAXIMUM LENGTH TO WIDTH RATIO OF 1:1.25.

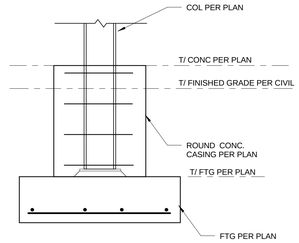
2 TYP. SLAB ON GRADE CONTROL JOINT

SCALE: 3/4" = 1'-0"



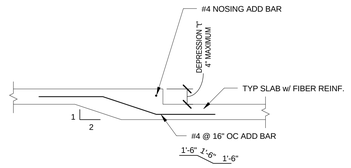
3 TYP. GRANDSTAND COLUMN FOOTING

S-S1.1 SCALE: 3/4" = 1'-0"



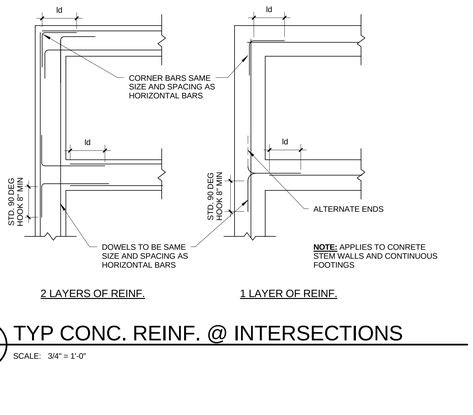
4 ENCASEMENT SECTION

SCALE: 3/4" = 1'-0"



5 SLAB DEPRESSION < 4"

SCALE: 1/2" = 1'-0"

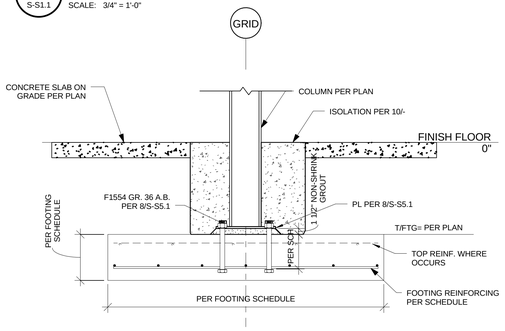


NOTE: APPLIES TO CONRETE STEM WALLS AND CONTINUOUS FOOTINGS

2 LAYERS OF REINF. 1 LAYER OF REINF.

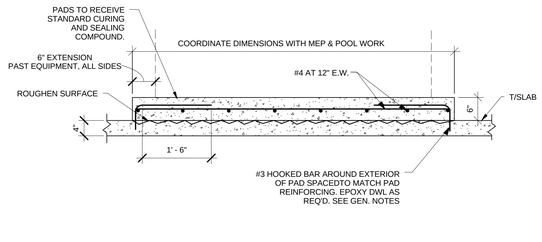
6 TYP CONC. REINF. @ INTERSECTIONS

SCALE: 3/4" = 1'-0"



7 TYPICAL INTERIOR FOOTING

S-S4.3 SCALE: 3/4" = 1'-0"

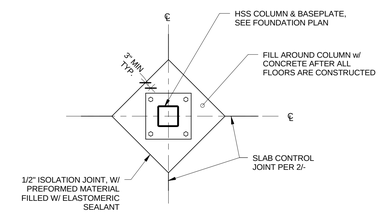


8 TYPICAL HOUSEKEEPING PAD

SCALE: 3/4" = 1'-0"

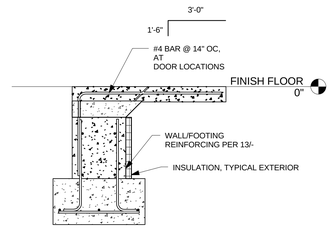
9 CMU WALL AT FOUNDATION - INTERIOR

S-S3.1 SCALE: 3/4" = 1'-0"



10 TYPICAL COLUMN ISOLATION DETAIL

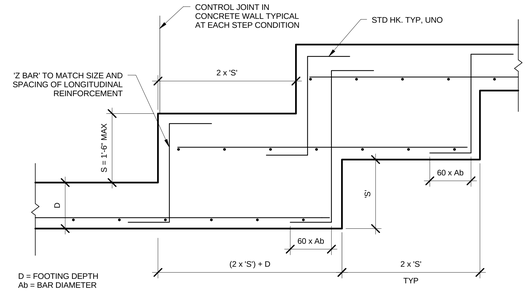
SCALE: 1/2" = 1'-0"



NOTE: COORDINATE DOOR LOCATIONS AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

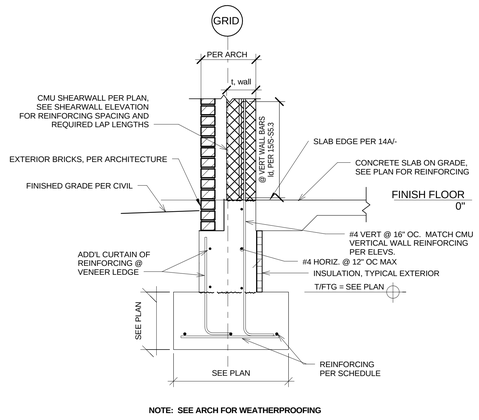
11 TYPICAL POUR-THRU AT DOOR

S-S1.1 SCALE: 3/4" = 1'-0"



12 TYPICAL STEPPED FOOTING

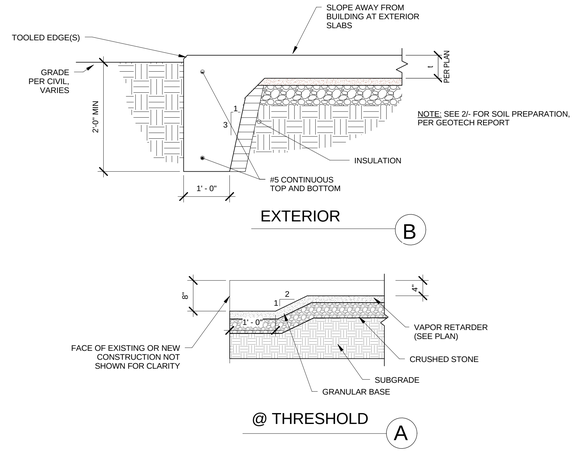
SCALE: 3/4" = 1'-0"



NOTE: SEE ARCH FOR WEATHERPROOFING

13 CMU WALL AT FOUNDATION - EXTERIOR

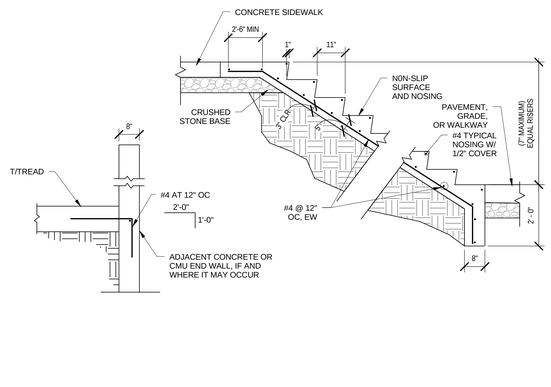
S-S1.1 SCALE: 3/4" = 1'-0"



NOTE: SEE 2/- FOR SOIL PREPARATION, PER GEOTECH REPORT

14 TYPICAL FLOOR SLAB PERIMETER

SCALE: 3/4" = 1'-0"



15 TYPICAL CONCRETE STEP ON GRADE

S-S4.3 SCALE: 1/2" = 1'-0"

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University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

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STADIUM FOUNDATION SECTIONS & DETAILS

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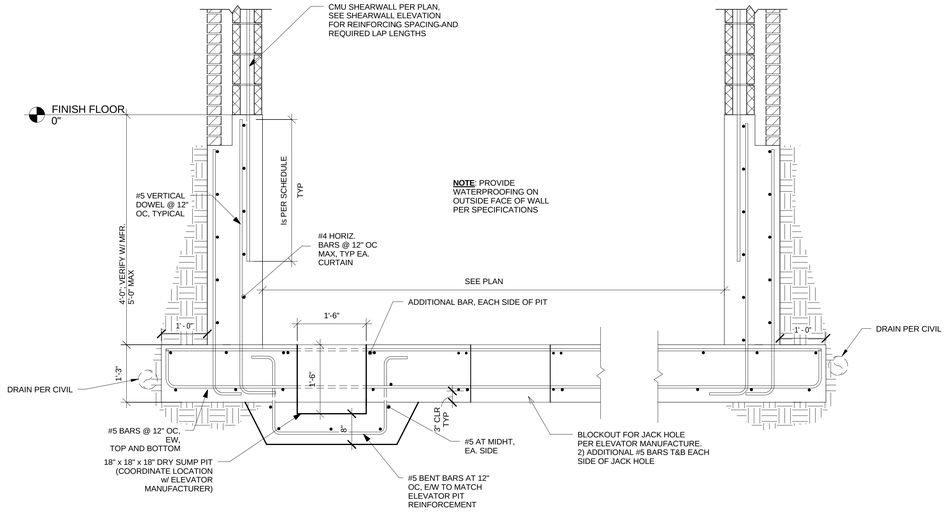
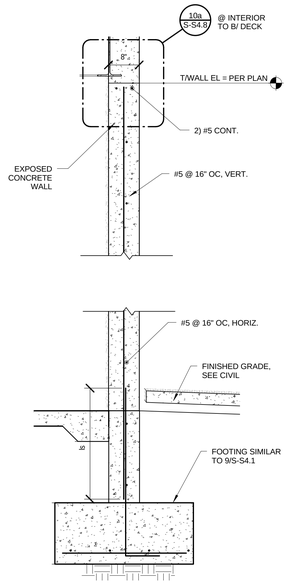


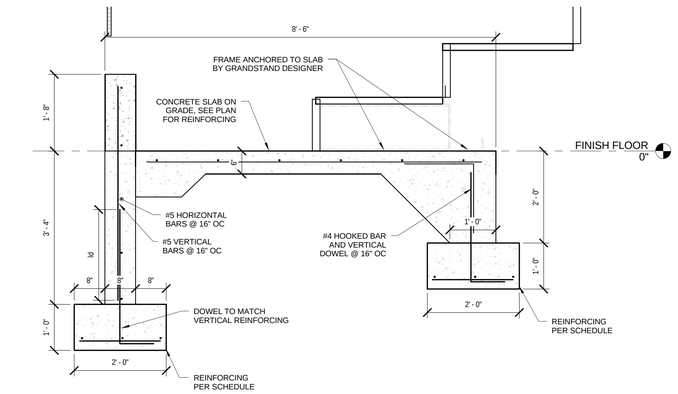
1 SECTION @ ELEVATOR PIT

S-S1.1 SCALE: 3/4" = 1'-0"



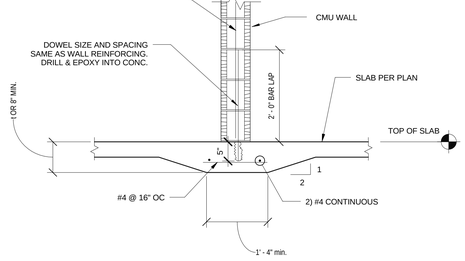
2 CONCRETE WALL

S-S1.1 SCALE: 3/4" = 1'-0"



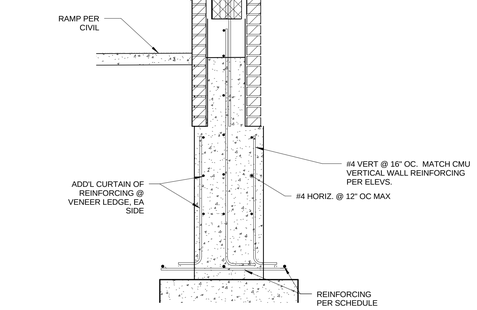
3 GRANDSTAND FRAMING TO SLAB CONNECTION

S-S1.1 SCALE: 3/4" = 1'-0"



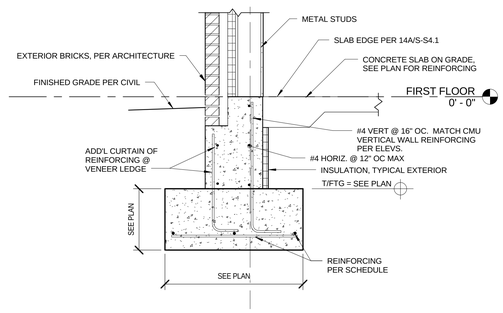
4 TYPICAL THICKENED SLAB @ CMU

SCALE: 3/4" = 1'-0"



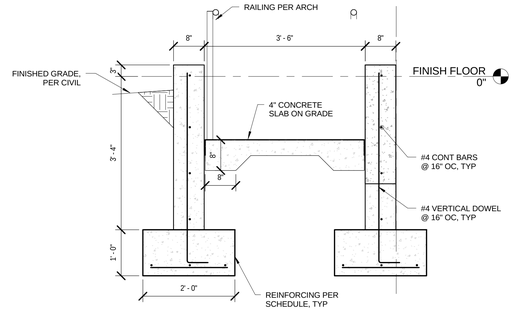
5 FOUNDATION AT RAMP WALL

S-S1.1 SCALE: 3/4" = 1'-0"



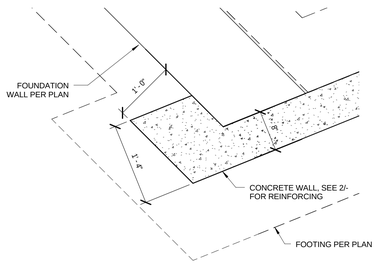
6 STAIR FOUNDATION

S-S1.1 SCALE: 3/4" = 1'-0"



7 CONCRETE RAMP

S-S1.1 SCALE: 3/4" = 1'-0"



8 CONCRETE WALL EXTENSION

S-S1.1 SCALE: 1" = 1'-0"

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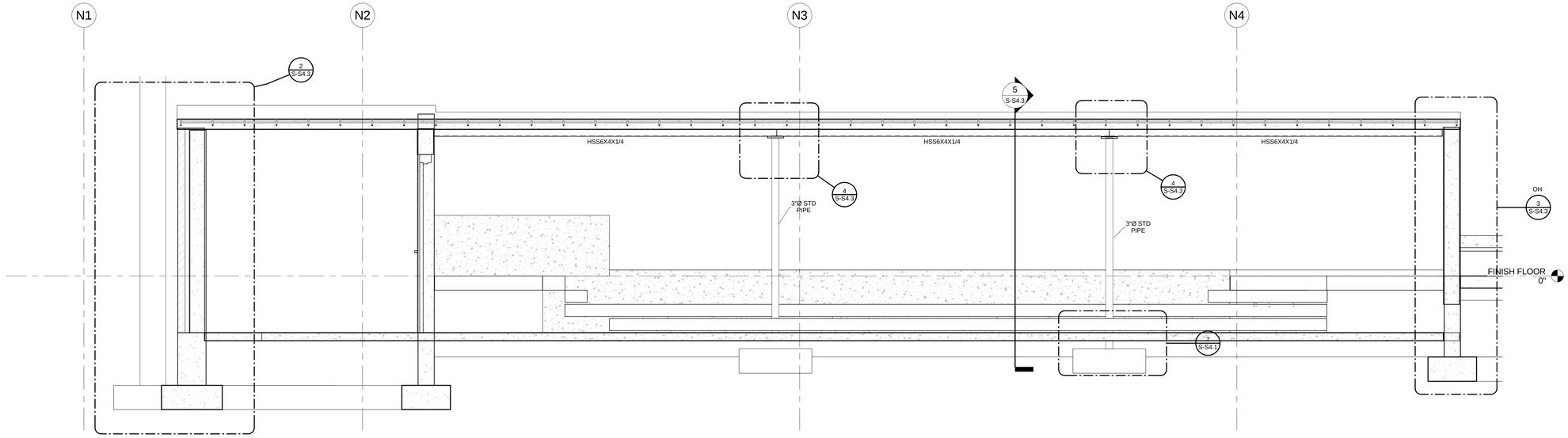
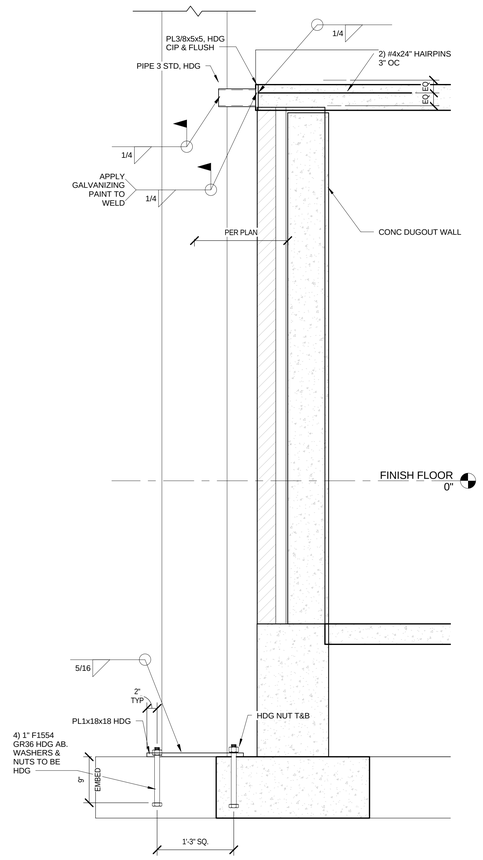
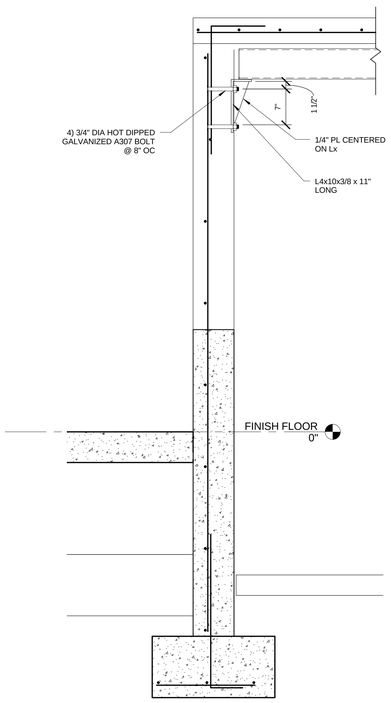


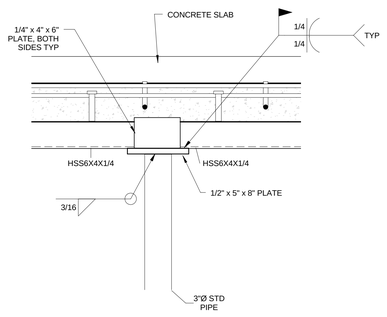
1 DUGOUT
S-S1.1 SCALE: 1/2" = 1'-0"



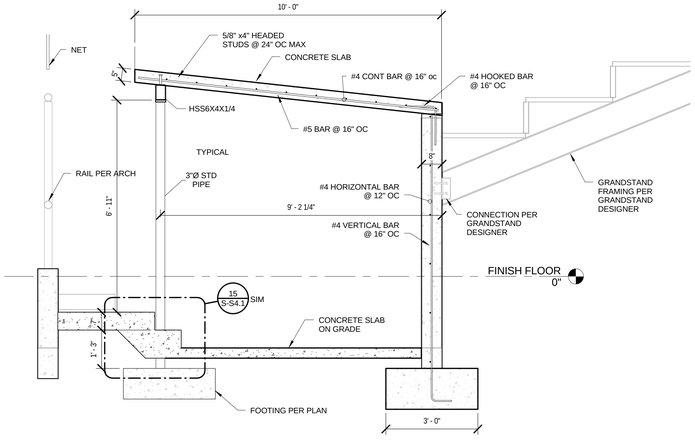
2 SECTION AT NETTING POLE
S-S3.2 SCALE: 1" = 1'-0"



3 SECTION THROUGH CONC SITE WALL
S-S1.1 SCALE: 1" = 1'-0"



4 DUGOUT COLUMN CONNECTION
S-S4.3 SCALE: 1 1/2" = 1'-0"



5 DUGOUT SECTION
S-S1.1 SCALE: 1/2" = 1'-0"

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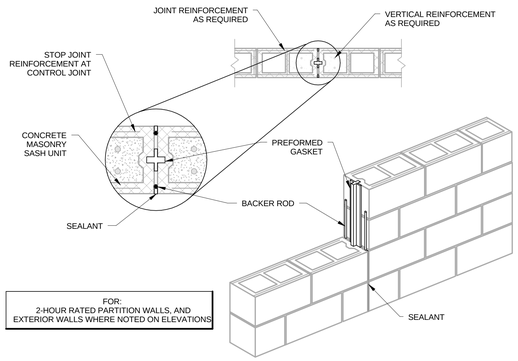
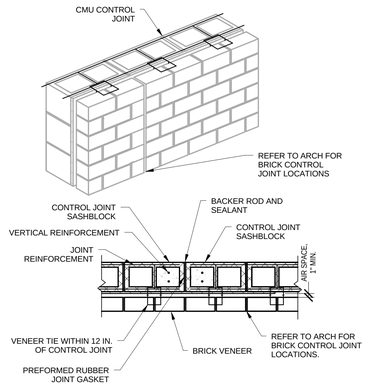


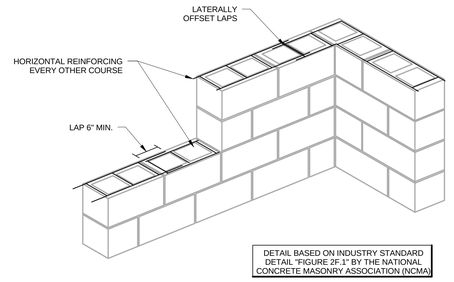
1 TYPICAL CMU CONTROL JOINT

SCALE: 3/4" = 1'-0"



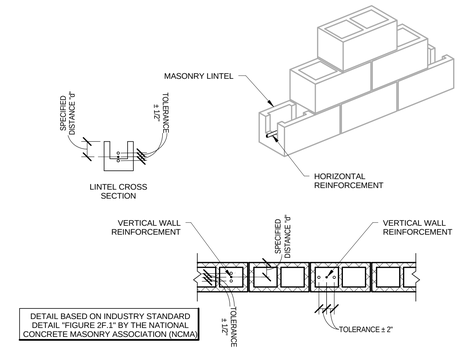
2 CMU AND BRICK COMBINED CONTROL JOINT

SCALE: 3/4" = 1'-0"



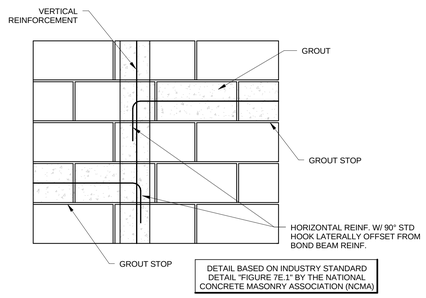
3 HORIZONTAL JOINT REINFORCEMENT

SCALE: 3/4" = 1'-0"



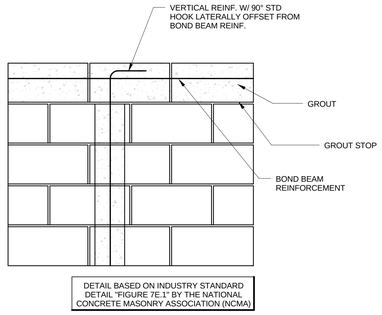
4 CMU REINFORCEMENT TOLERANCE

SCALE: 3/4" = 1'-0"



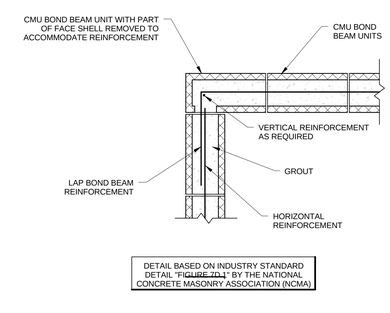
5 INTERSECTION OF REINFORCEMENT AT BOND BEAM

SCALE: 1" = 1'-0"



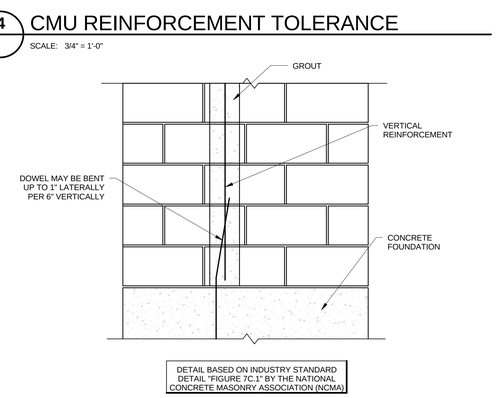
6 INTERSECTION OF REINFORCEMENT AT T/WALL

SCALE: 1" = 1'-0"



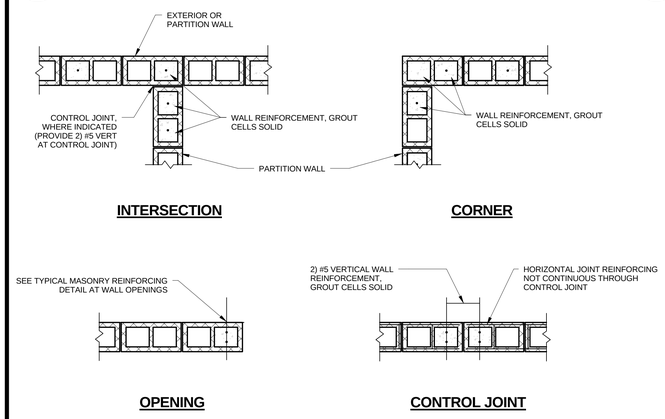
7 CMU CORNER REINFORCEMENT

SCALE: 1" = 1'-0"



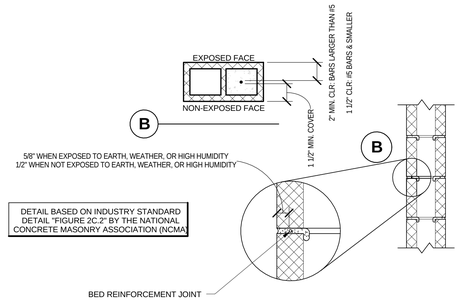
8 CMU REINFORCEMENT BEND TOLERANCE

SCALE: 1" = 1'-0"



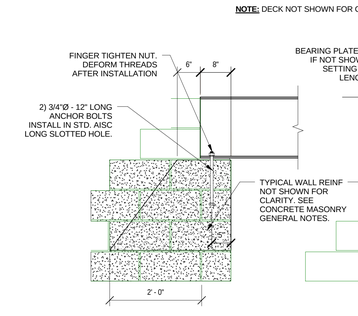
9 MASONRY VERTICAL REINFORCING

SCALE: 3/4" = 1'-0"



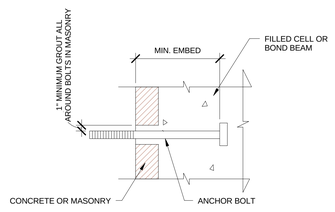
10 BED JOINT REINFORCEMENT

SCALE: 1" = 1'-0"



11 BEAM BEARING ON MASONRY

S-S2.1 SCALE: 3/4" = 1'-0"

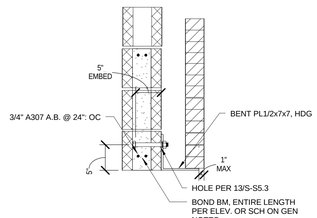


NOTE: MINIMUM BOLT SPACING SHALL BE 12 x BOLT DIAMETERS WITH A MINIMUM EDGE DISTANCE OF 6 DIAMETERS.(CONTRACTOR OPTION TO USE EPOXY GROUTED ANCHORS IN LIEU OF C.I.P. ANCHORS, WHERE AND WHEN APPROVED BY THE ENGINEER OF RECORD)

BOLT SIZE	EMBEDMENT
1/2"	4"
5/8"	5"
3/4"	6"
7/8"	6"

13 TYPICAL WALL BOLT EMBEDMENT

SCALE: 1/2" = 1'-0"



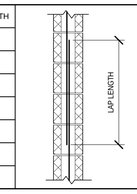
NOTE: SEE GENERAL NOTES FOR BRICK LEDGERS SPANNING LESS THAN 8'-0".

14 LONG-SPAN BRICK SUPPORT

S-S3.2 SCALE: 1" = 1'-0"

LAP SPLICE LENGTHS FOR CMU WALL REINFORCEMENT
(GRADE 60 UNCOATED BARS IN FULLY GROUTED CELLS)

BAR SIZE	LAP LENGTH	DEVELOPMENT LENGTH
# 3	2-3"	1'-6"
# 4	3'-0"	2'-0"
# 5	3'-9"	2'-11"
# 6	4'-6"	5'-5"
# 7	5'-3"	7'-5"
# 8	6'-0"	11'-2"
# 9	7'-7"	14'-3"



NOTES:
1. LAP SPLICE LENGTHS ARE TO BE INCREASED BY 50% WHEN EPOXY COATED REINFORCEMENT IS USED.
2. #7 AND LARGER VERT. BARS ARE REQUIRED TO BE SPLICED BY MECHANICAL COUPLERS WITHIN WALLS. NO CONSTRUCTION JOINT SHALL OCCUR WITHIN A GROUT LIFT/CONSTRUC. JOINT.

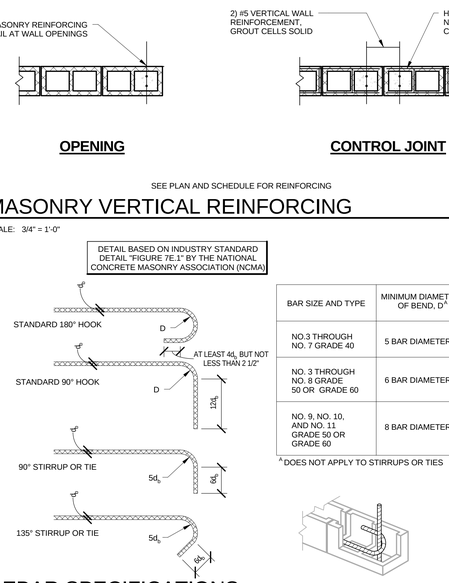
15 CMU LAP SPLICES & DEVELOPMENT LENGTH

SCALE: 3/4" = 1'-0"

DETAIL BASED ON INDUSTRY STANDARD DETAIL "FIGURE 7D.1" BY THE NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA)

BAR SIZE AND TYPE	MINIMUM DIAMETER OF BEND, D^A
NO.3 THROUGH NO. 7 GRADE 40	5 BAR DIAMETERS
NO. 3 THROUGH NO. 8 GRADE 50 OR GRADE 60	6 BAR DIAMETERS
NO. 9, NO. 10, AND NO. 11 GRADE 50 OR GRADE 60	8 BAR DIAMETERS

^A DOES NOT APPLY TO STIRRUPS OR TIES



12 REBAR SPECIFICATIONS

SCALE: 3/4" = 1'-0"

SEE PLAN AND SCHEDULE FOR REINFORCING

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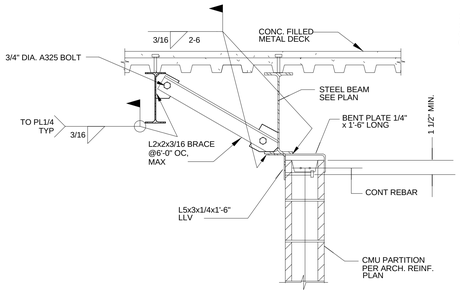
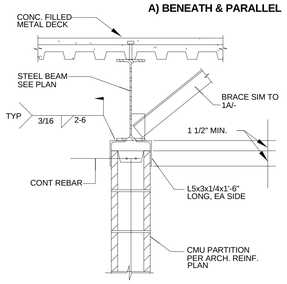
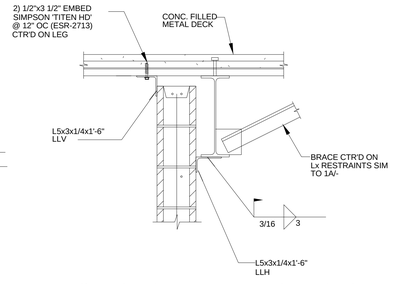


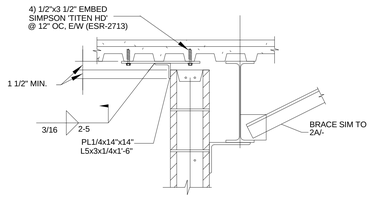
A) BENEATH & PARALLEL



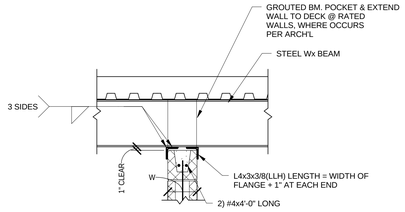
B) CENTERED BENEATH BEAM



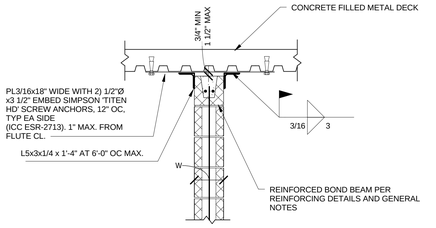
C) ADJACENT & PARALLEL: PERPENDICULAR TO DECK



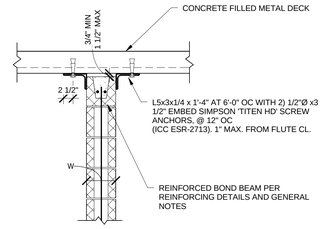
D) ADJACENT & PARALLEL: PARALLEL TO DECK



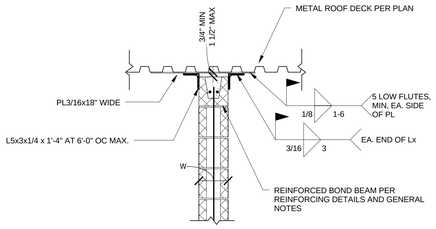
E) PERPENDICULAR TO BEAM



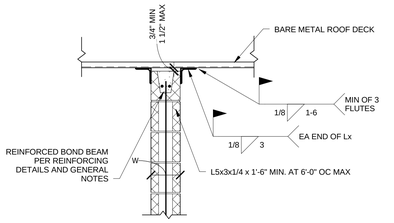
PARALLEL TO CONC. FILELD DECK



PERPENDICULAR TO CONC. FILLED DECK



PARALLEL TO ROOF DECK



PERPENDICULAR TO ROOF DECK

1 TYP. TOP OF CMU PARTITION BRACING @ Wx BEAM

S-S2.1 SCALE: 1" = 1'-0"

2 TYPICAL CMU PARTITION BRACING @ DECK

SCALE: 3/4" = 1'-0"

3 SECTION @ SIGNAGE FRAME

S-S2.2 SCALE: 1" = 1'-0"

CONSTRUCTION DOCUMENTS

0 1/2" 1"

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

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DATE 5/30/12
STADIUM FRAMING SECTIONS
DRAWING NO. **S-S4.5** | BID PKG.

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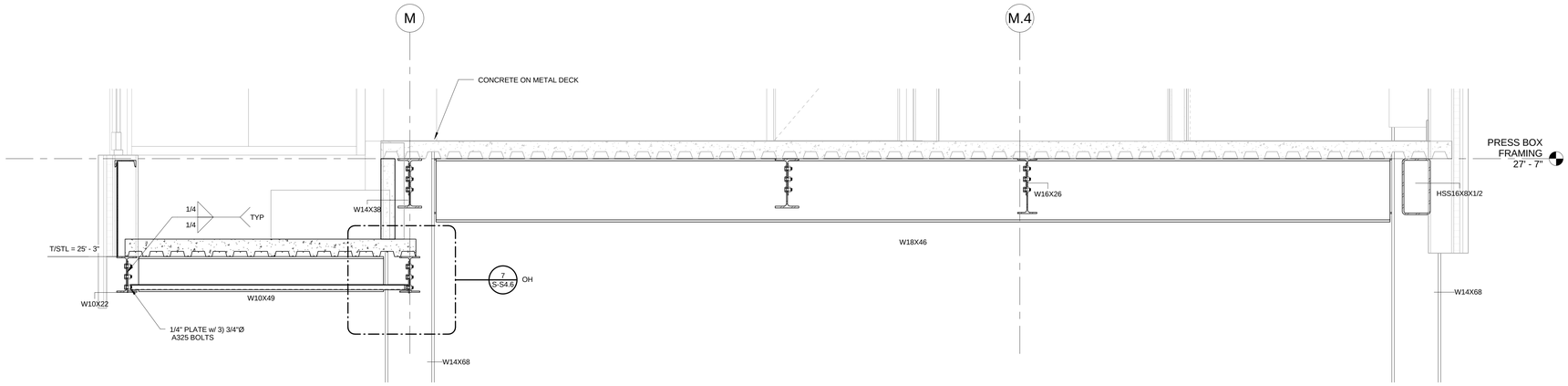
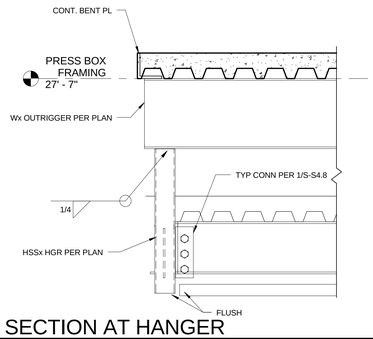


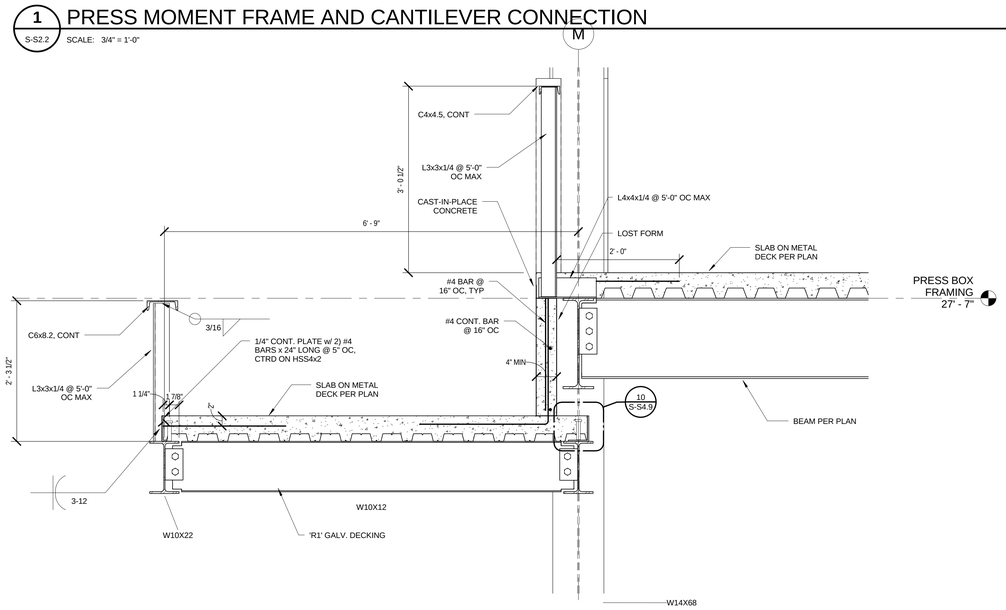
1 S-S2.2 PRESS MOMENT FRAME AND CANTILEVER CONNECTION

SCALE: 3/4" = 1'-0"



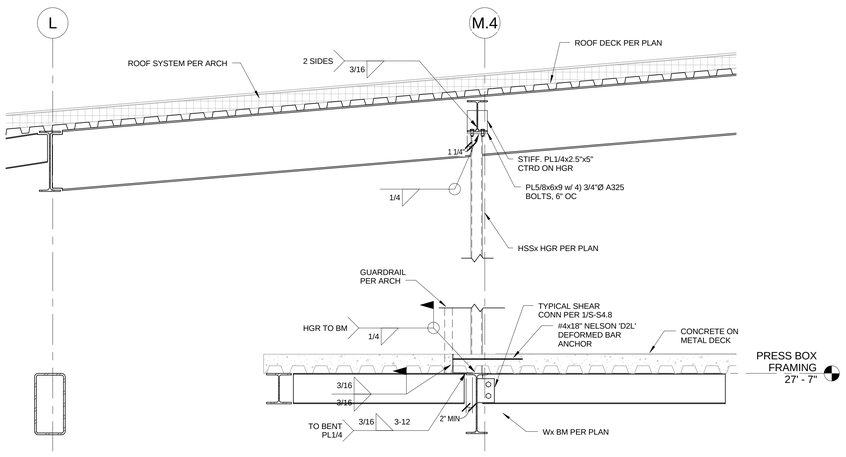
2 S-S2.2 SECTION AT HANGER

SCALE: 1" = 1'-0"



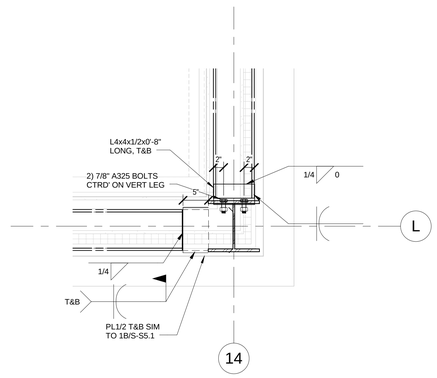
3 S-S3.1 PRESS STEP

SCALE: 1" = 1'-0"



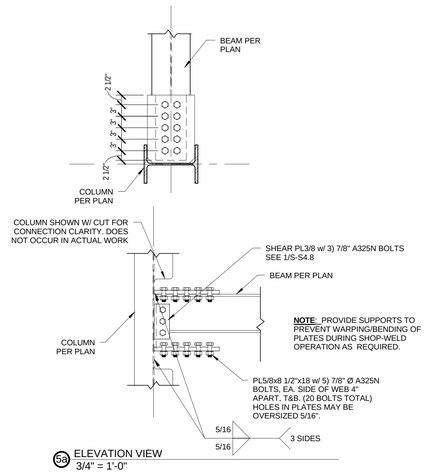
4 S-S2.2 PRESS STAIR SUPPORT

SCALE: 3/4" = 1'-0"



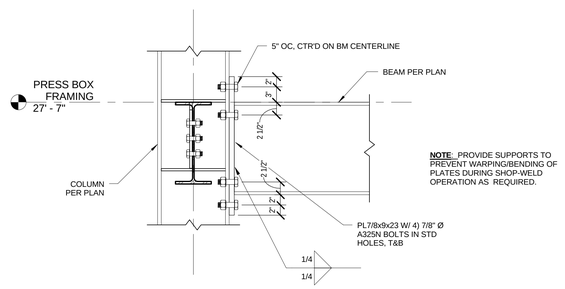
5 S-S2.1 HSSx BM TO Wx COL

SCALE: 1" = 1'-0"



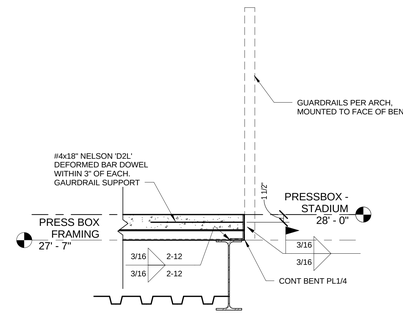
6 S-S2.2 WEAK AXIS MOMENT CONNECTION AT PRESS BOX

SCALE: 3/4" = 1'-0"



7 S-S2.2 STRONG AXIS MOMENT CONNECTION AT PRESS BOX

SCALE: 1" = 1'-0"



8 S-S2.2 EDGE OF DECK @ GUARDRAIL

SCALE: 1" = 1'-0"

CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

0 1/2" 1"

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Renovate/Upgrade Softball Complex

PROJECT NUMBER: 2338.0

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REVISIONS		
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STADIUM PRESS FRAMING SECTIONS

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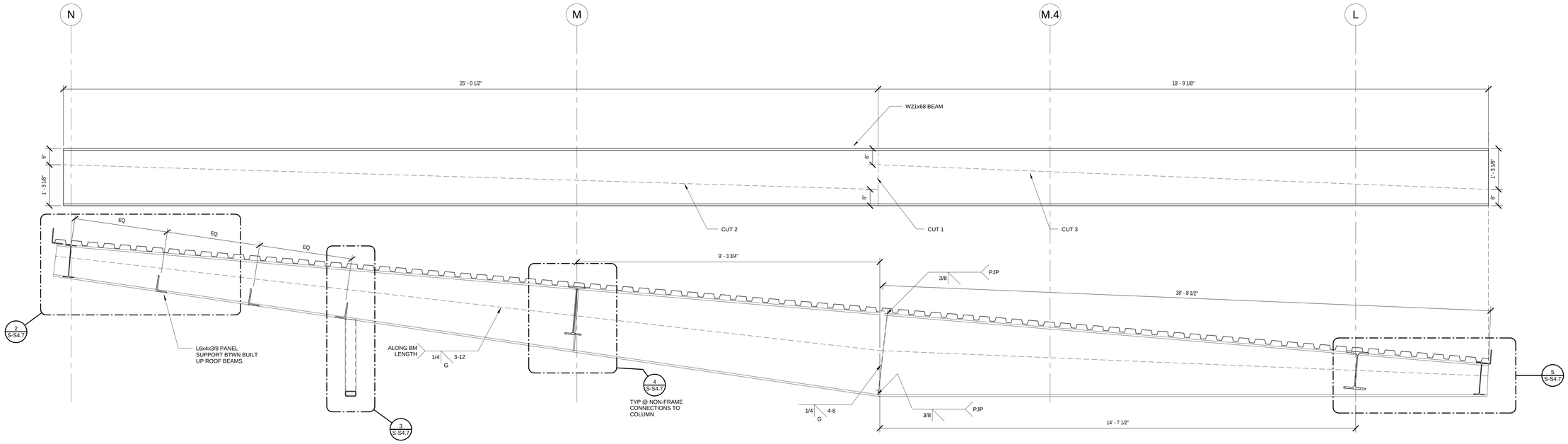
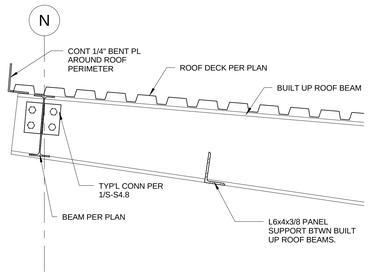


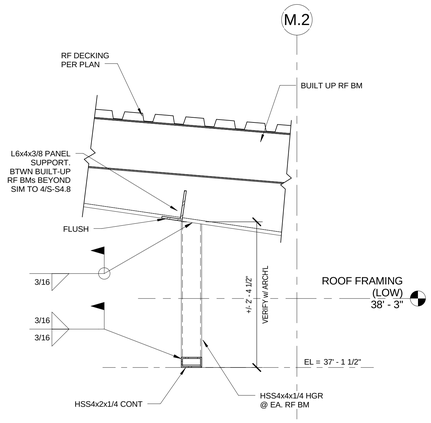
1 S-S2.2 STADIUM ROOF GIRDER BEAM

SCALE: 3/4" = 1'-0"



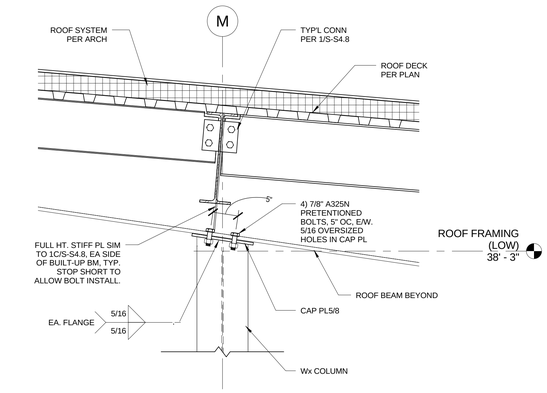
2 S-S4.7 ROOF EXTENSION DETAIL

SCALE: 1" = 1'-0"



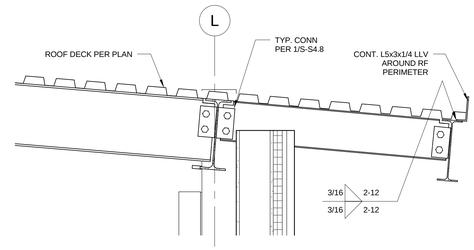
3 S-S3.1 STANCHION @ HEAD OF GLAZING

SCALE: 1" = 1'-0"



4 S-S3.1 ROOF BEAM TO COL CONN (NON-FRAME)

SCALE: 1" = 1'-0"



5 S-S3.1 ROOF CANTILEVER BEAM

SCALE: 1" = 1'-0"

CONSTRUCTION DOCUMENTS

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

0 1/2" 1"

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STADIUM ROOF
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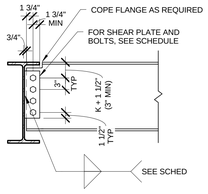
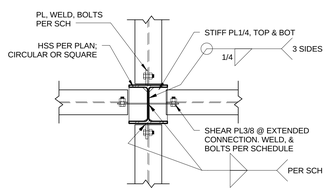
SHEAR PLATE AND BOLT SCHEDULE				
BEAM SIZE	NO. & SIZE OF BOLTS REQUIRED	PLATE THICKNESS	WELD SIZE	CAPACITY (ASD)
W8	2) 7/8"Ø	1/4"	3/16"	17.4k
W10	2) 7/8"Ø	1/4"	1/4"	17.4k
W12	3) 7/8"Ø	1/4"	1/4"	26.1k
W14	3) 7/8"Ø	1/4"	1/4"	26.1k
W16	4) 7/8"Ø	5/16"	1/4"	43.5k
W18	5) 7/8"Ø	5/16"	1/4"	54.4k
W21	6) 7/8"Ø	3/8"	1/4"	78.3k
W24	7) 7/8"Ø	3/8"	1/4"	91.4k
W27	8) 7/8"Ø	3/8"	1/4"	104k

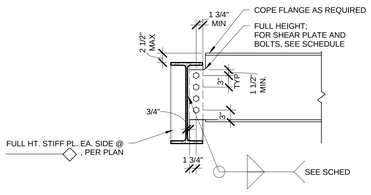
CONNECTION NOTES:
1. ALL BOLTS TO BE ASTM A325-N AND FULLY PRETENSIONED PER AISC STANDARDS EXCEPT AS DESCRIBED IN NOTE 2.
2. BOLTS IN BEAM TO BEAM CONNECTIONS SHALL BE TIGHTENED TO AISC "SNUG TIGHT" CONDITION UNLESS NOTED OTHERWISE ON PLAN.
3. CONNECTION PLATES TO HAVE AISC STANDARD ROUND HOLES UNLESS NOTED OTHERWISE.
4. ALL CONNECTION PLATE MATERIAL TO BE A36.
5. 'K' PER AISC, LARGER OF THE TWO CONNECTING BEAMS.
6. PROVIDE PL 3/8" MIN AT ALL DRAG CONNECTIONS, AS NOTED ON PLAN.



Ⓐ BEAM TO BEAM
3/4" = 1'-0"

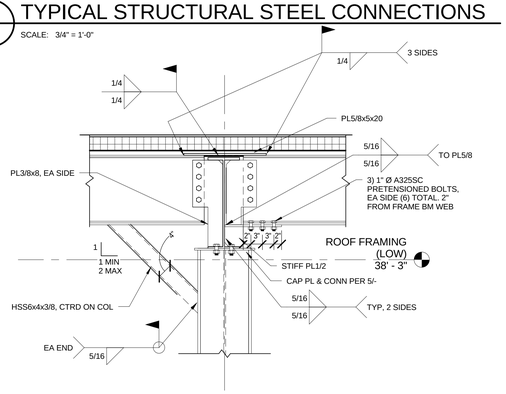


Ⓑ PLAN AT Wx COLUMN
3/4" = 1'-0"

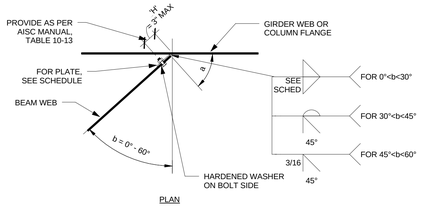


Ⓒ BEAM TO DEPRESSED GIRDER
3/4" = 1'-0"

1 TYPICAL STRUCTURAL STEEL CONNECTIONS
S-S3.2 SCALE: 3/4" = 1'-0"



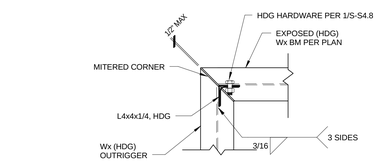
2 SKEWED BEAM CONNECTION
S-S3.2 SCALE: 3/4" = 1'-0"



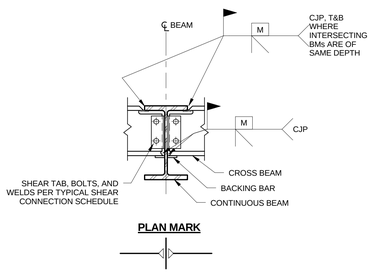
PLAN

SKEWED BEAM CONNECTION SCHEDULE: (SEE AISC TABLE 10-13)				
BEAM SIZE	NO. & SIZE OF BOLTS REQUIRED	PLATE THICKNESS	WELD FOR 0°<b<17°	WELD FOR 17°<b<30°
W8	SEE TYPICAL BOLT CONNECTION SCHEDULE	5/16"	1/4 3/8	1/4 1/2 45°
W10		5/16"		
W12		5/16"		
W14		5/16"		
W16		3/8"		
W18		3/8"		
W21		3/8"		
W24		3/8"		
W27		3/8"		
W30		3/8"		

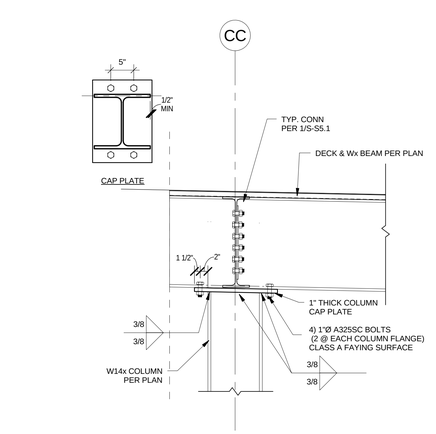
NOTES:
1. FILLET WELD SIZE S = PLATE THICKNESS
2. FOR BEVEL WELDS, BEVEL END OF CONNECTION PLATE TO MAINTAIN a AT 45°
3. DO NOT INCREASE DIM X WITHOUT ENGINEER'S APPROVAL.



3 TYPICAL BEAM TO BEAM MOMENT CXN
SCALE: 3/4" = 1'-0"



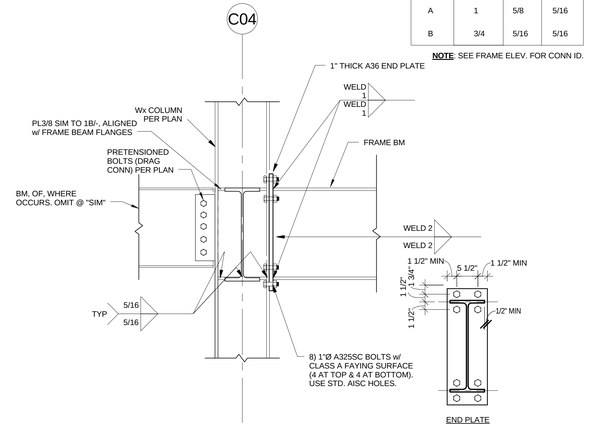
PLAN MARK



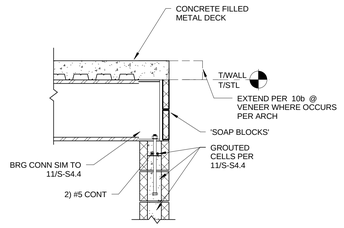
4 TYPICAL ROOF OPENING FRAMING
SCALE: 1/2" = 1'-0"

CONNECTION SCHEDULE			
CONN ID	PL THICK.	WELD 1	WELD 2
A	1	5/8	5/16
B	3/4	5/16	5/16

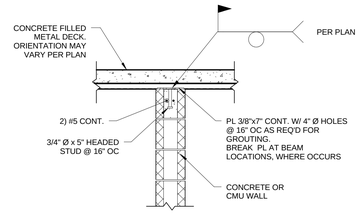
NOTE: SEE FRAME ELEV. FOR CONN ID.



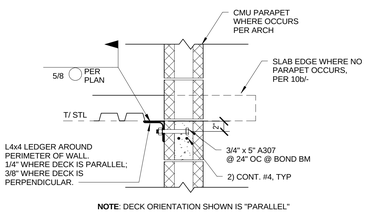
5 LONG BAY - FRAME CONN AT RF
S-S3.2 SCALE: 3/4" = 1'-0"



6 TYPICAL FRAME CONN AT FLOOR
S-S3.2 SCALE: 3/4" = 1'-0"



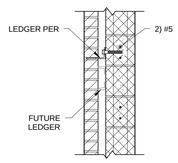
7 SHORT BAY - RF FRAME DETAIL
S-S3.2 SCALE: 3/4" = 1'-0"



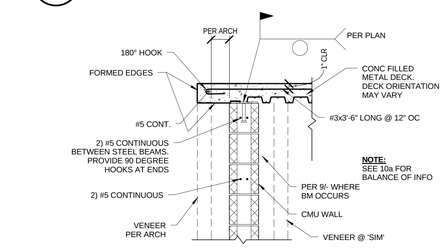
NOTE: DECK ORIENTATION SHOWN IS "PARALLEL"

8 BASE PLATE & ANCHOR RODS
SCALE: 1" = 1'-0"

9 TYPICAL BEAM TO CMU WALL
S-S2.1 SCALE: 3/4" = 1'-0"



10a TYPICAL DECK TO WALL @ INTERIOR
S-S2.1 SCALE: 3/4" = 1'-0"

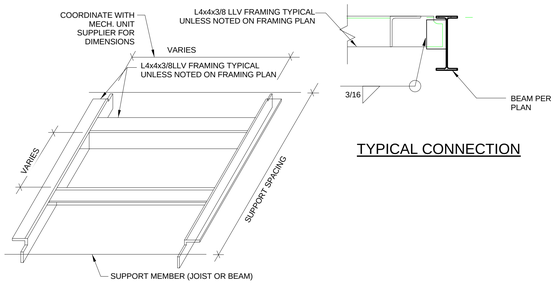


10b TYPICAL DECK AT WALL @ EXTERIOR
S-S2.1 SCALE: 3/4" = 1'-0"

11 LEDGER ANGLE
S-S2.1 SCALE: 1" = 1'-0"

12 MITERED BMs AT EXPOSED CORNERS
SCALE: 1" = 1'-0"

13 BRICK LEDGER AT ELEVATOR
S-S2.1 SCALE: 3/4" = 1'-0"



TYPICAL CONNECTION

CONSTRUCTION DOCUMENTS

0 1/2" 1"
SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

COMM. NO.	2012.030.00
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DATE	5/30/12
STADIUM STEEL DETAILS	
DRAWING NO.	BID PKG.
S-S4.8	

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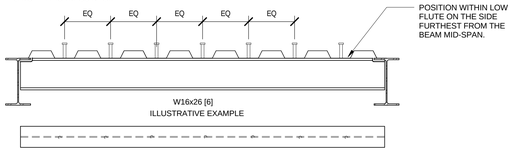
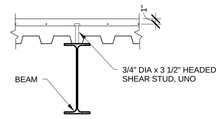
1. SHEAR STUDS SHALL BE 3/4"Ø HEADED STUDS WITH FINISHED STUD LENGTH OF 3 1/2".
2. PROVIDE STUDS AT MAXIMUM SPACING OF 18" OC.
3. STUDS MUST BE WELDED TO BEAM WITH A STUD WELDING GUN AS REQUIRED BY AWS D1.1, STICK WELDING IS NOT PERMITTED FOR HEADED STUDS. STUDS SHALL BE WELDED IN FIELD THROUGH A SINGLE THICKNESS DECK.



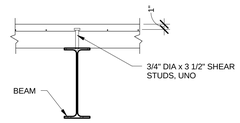
1 HEADED STUD ANCHORING NOTES

SCALE: 3/4" = 1'-0"



NOTE: DECK RIB VALLEY TO BE CENTERED OVER BEAM WHERE POSSIBLE OR DECK MUST BE SPLIT FULL LENGTH OF BEAM TO PROVIDE CONCRETE HAUNCH WITH A MINIMUM WIDTH OF 4 1/2"

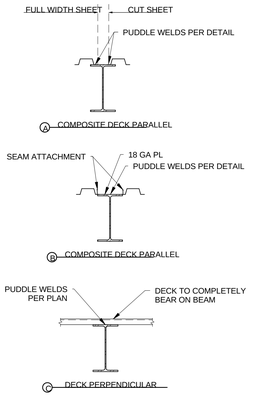
DECK PARALLEL TO BEAM



DECK PARALLEL TO BEAM

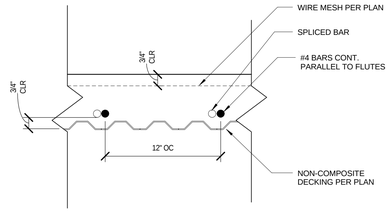
2 TYPICAL SECTION AT COMPOSITE BEAM

SCALE: 1" = 1'-0"



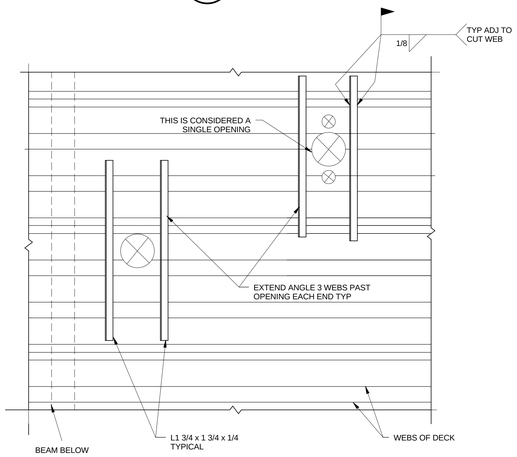
3 DECK ALLIGNMENT / BEARING DETAIL

SCALE: 3/4" = 1'-0"



4 'S1' SECTION VIEW

SCALE: 3" = 1'-0"

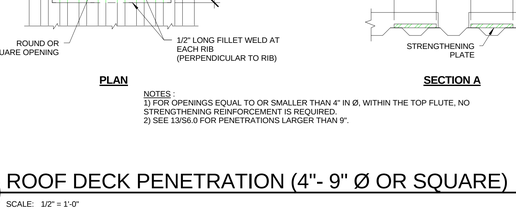


NOTES:

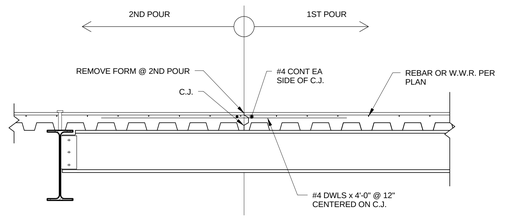
1. HOLES LESS THAN 6" IN DIAMETER AND CUTTING NO MORE THAN 1 WEB NEED NO REINFORCING.
2. DO NOT CUT MORE THAN 2 ADJACENT WEBS.
3. ANGLES SHALL BE PLACED ON TOP OF DECK. ALTERNATELY THEY CAN BE PLACED ON BOTTOM OF DECK.
4. IF DIMENSIONS "A" AND "B" ARE LESS THAN 4D1, 4D2, OR 32" WHICHEVER IS LARGER, THE OPENING GROUP SHALL BE CONSIDERED AS A SINGLE HOLE, AND MUST BE REINFORCED AS REQUIRED FOR THE LARGER OPENING.
5. IF DIMENSION "A" IS GREATER THAT 4D1, 4D2, OR 32" WHICHEVER IS LARGER, THEN THERE IS NO RESTRICTION ON DIMENSION "B".
6. IF DIMENSION "B" IS GREATER THAN 4D1, 4D2, OR 32" WHICHEVER IS LARGER, THEN THERE IS NO RESTRICTION ON DIMENSION "A".
7. WHEN MAXIMUM DIMENSION OF AN OPENING OR OPENING GROUP EXCEEDS 12" USE DETAIL 4/-.
8. SUBMIT LOCATIONS OF PENETRATIONS PRIOR TO POURING CONC, FOR REVIEW.

5 CONSTRUCTION JOINT FOR SLAB

SCALE: 1" = 1'-0"



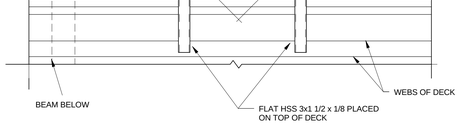
JOINT PERPENDICULAR TO DECK



JOINT PARALLEL TO DECK

6 CONCRETE OVER METAL DECK PENETRATION

SCALE: 3/4" = 1'-0"

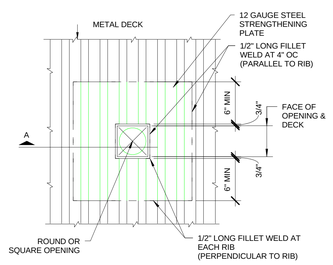


NOTES:

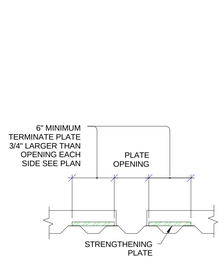
1. IF THE OPENING OR GROUP OF OPENINGS OCCURS IN ONE DECKING UNIT, THE OPENING OR OPENING GROUP MAY BE CUT PRIOR TO POURING OF CONCRETE.
2. IF, AS SHOWN IN THE DETAIL ABOVE, THE OPENING OR OPENING GROUP CUTS THROUGH TWO DECKING UNITS, THE DECKING SHALL NOT BE CUT UNTIL CONCRETE HAS BEEN PLACED AND CURED FOR A MINIMUM OF 7 DAYS. AT THE TIME OF POURING, SUITABLE SLEEVES OR BULKHEADS SHALL BE PLACED AROUND THE OPENING.
3. ADD REBAR AT CORNER OF OPENING.
4. WHEN THE MAXIMUM DIMENSION OF AN OPENING OR OPENING GROUP EXCEEDS 24", SEE PLANS FOR THE OPENING FRAMING.

7 FLOOR DECK DECK SLEEVE DETAIL

SCALE: 3/4" = 1'-0"

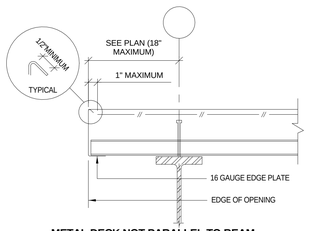


PLAN



SECTION A

NOTES :
1) FOR OPENINGS EQUAL TO OR SMALLER THAN 4" IN Ø, WITHIN THE TOP FLUTE, NO STRENGTHENING REINFORCEMENT IS REQUIRED.
2) SEE 13/S6.0 FOR PENETRATIONS LARGER THAN 9".



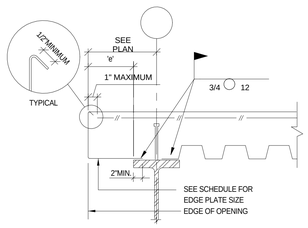
METAL DECK NOT PARALLEL TO BEAM
CANTILEVER NOT MORE THAN 18"
TYPICAL SLAB EDGE AT SLAB OPENINGS

SCHEDULE FOR EDGE PLATE

SLAB THICKNESS (INCHES) \ 'e' (OVERHANG IN INCHES)	0 "	1 "	2 "	3 "	4 "	5 "	6 "	7 "	8 "	9 "	10"	11"	12"
4 1/2	20	20	20	18	18	16	16	14	12	12	12	10	10
4 5/8 TO 5 1/4	20	18	18	16	16	14	14	12	12	12	10	10	
5 3/8 TO 6 1/4	18	18	16	14	14	12	12	12	12	10	10		
6 3/8 TO 7 1/2	16	14	14	12	12	12	12	10	10	10			
7 5/8 TO 9	14	12	12	12	10	10	10						
9 1/2 TO 11	12	10	10	10									
11 1/4 TO 12	10	10											

INDICATES GAUGE SIZE (TYPICAL)

NOTE: THIS EDGE PLATE IS NOT DESIGNED TO SUPPORT LOADS IMPOSED BY CURTAINWALL, GUARD RAILS, VENEER, ETC. ATTACHMENTS.
A CONT. 1/4" BENT PLATE SIM TO 3/S6.3 SHALL BE UTILIZED FOR SUCH CONDITIONS, WITH PRIOR APPROVAL FROM THE ENGINEER OF RECORD.



METAL DECK PARALLEL TO BEAM
OVERHANG - NOT MORE THAN 12"

SCHEDULE FOR EDGE PLATE

SLAB THICKNESS (INCHES) \ 'e' (OVERHANG IN INCHES)	0 "	1 "	2 "	3 "	4 "	5 "	6 "	7 "	8 "	9 "	10"	11"	12"
4 1/2	20	20	20	18	18	16	16	14	12	12	12	10	10
4 5/8 TO 5 1/4	20	18	18	16	16	14	14	12	12	12	10	10	
5 3/8 TO 6 1/4	18	18	16	14	14	12	12	12	12	10	10		
6 3/8 TO 7 1/2	16	14	14	12	12	12	12	10	10	10			
7 5/8 TO 9	14	12	12	12	10	10	10						
9 1/2 TO 11	12	10	10	10									
11 1/4 TO 12	10	10											

INDICATES GAUGE SIZE (TYPICAL)

NOTE: THIS EDGE PLATE IS NOT DESIGNED TO SUPPORT LOADS IMPOSED BY CURTAINWALL, GUARD RAILS, VENEER, ETC. ATTACHMENTS.
A CONT. 1/4" BENT PLATE SIM TO 3/S6.3 SHALL BE UTILIZED FOR SUCH CONDITIONS, WITH PRIOR APPROVAL FROM THE ENGINEER OF RECORD.

8 ROOF DECK PENETRATION (4"- 9" Ø OR SQUARE)

SCALE: 1/2" = 1'-0"

9 TYPICAL PERPENDICULAR SLAB EDGE AT SLAB OPENINGS

SCALE: 1/2" = 1'-0"

10 TYPICAL SLAB EDGE AT SLAB OPENINGS

S-S4.6

SCALE: 1/2" = 1'-0"

CONSTRUCTION DOCUMENTS

0 1/2" 1"

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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University of Kentucky
Renovate/Upgrade Softball Complex
PROJECT NUMBER: 2338.0
SOUTH CAMPUS
ALUMNI DR./COLLEGE WAY
LEXINGTON, KENTUCKY

REVISIONS		
#	Description	Date

COMM. NO. 2012.030.00
MADE BY AEM | CHECKED
DATE 5/30/12

STADIUM STEEL DECK DETAILS

DRAWING NO. S-S4.9 | BID PKG.

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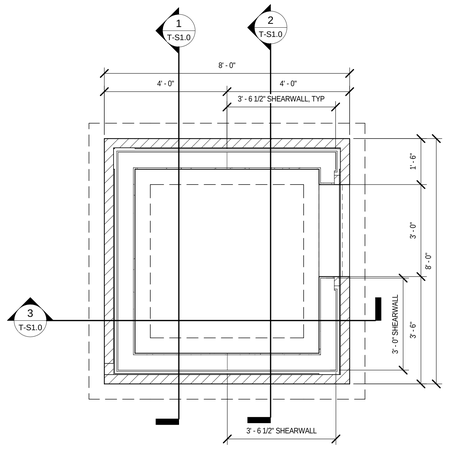
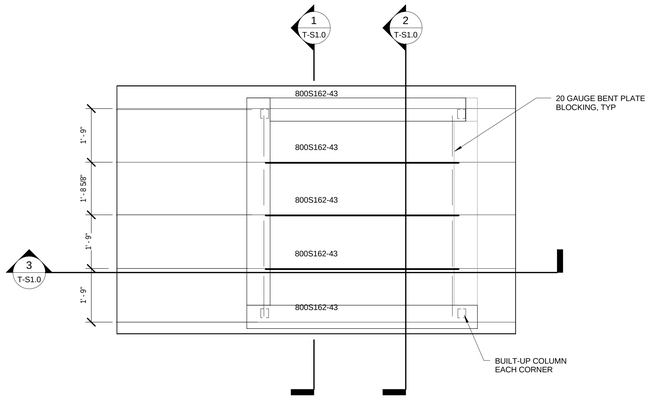


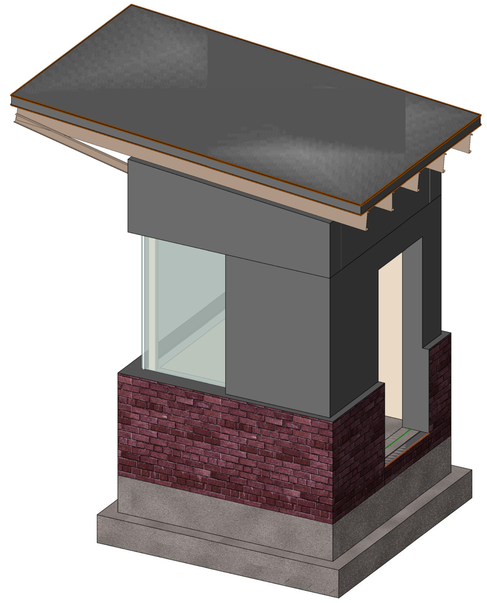
TICKET BOOTH FOUNDATION PLAN

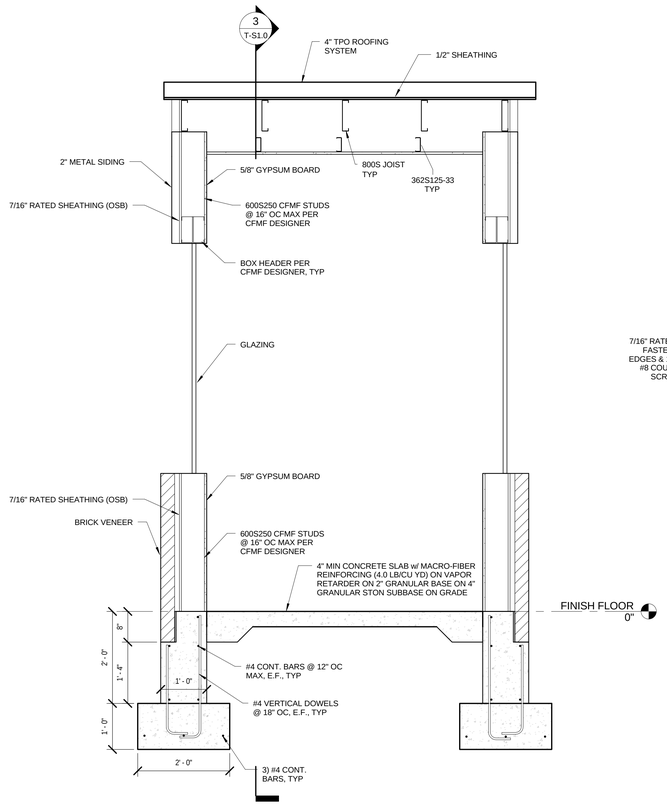
SCALE: 1/2" = 1'-0"



TICKET BOOTH FRAMING PLAN

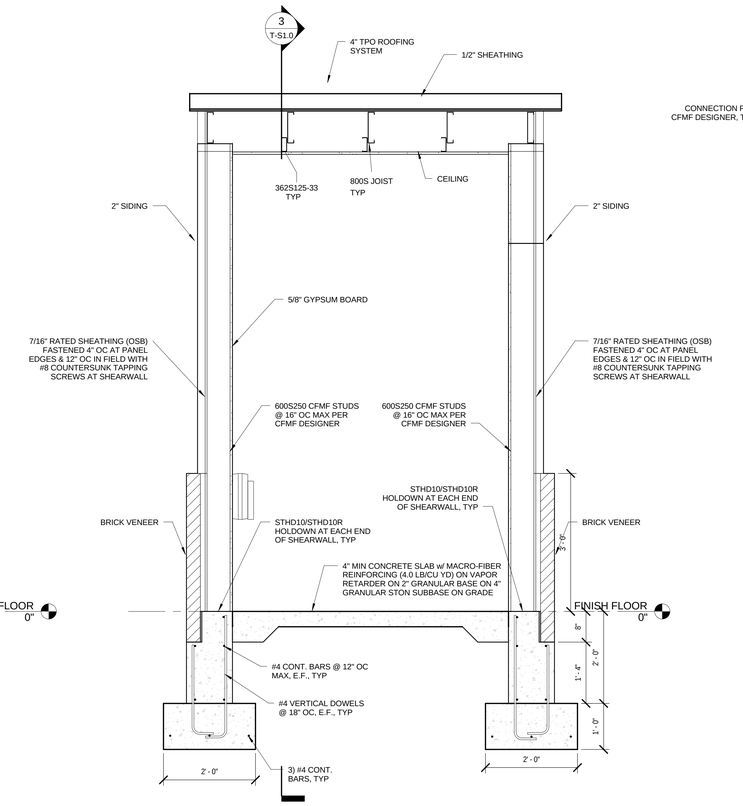
SCALE: 1/2" = 1'-0"





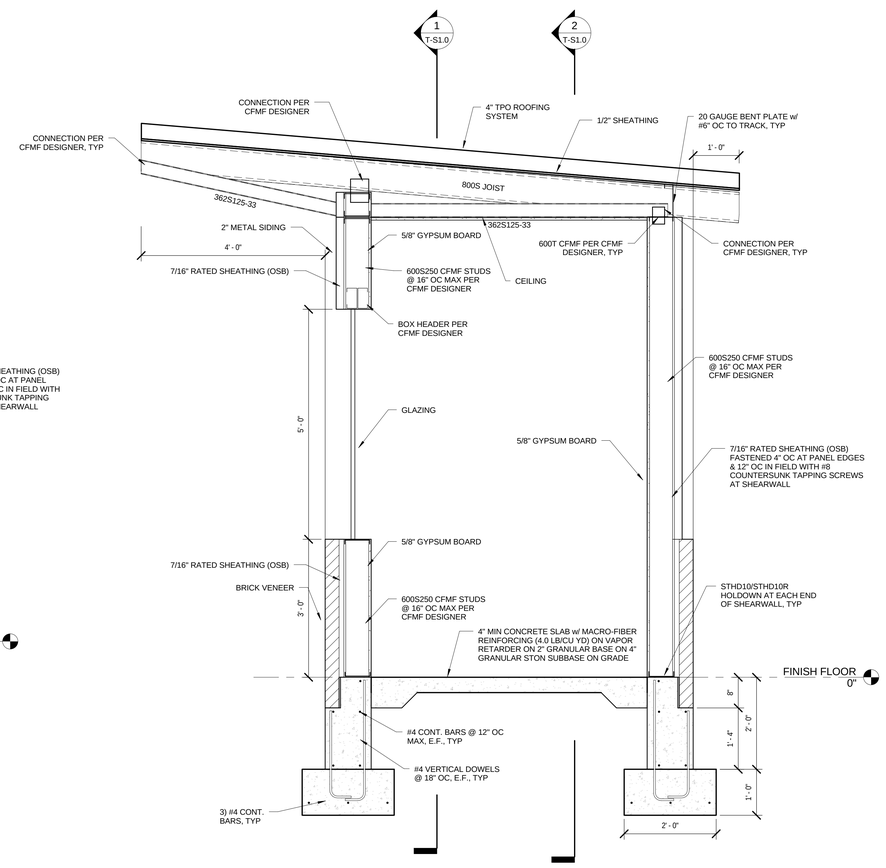
1 TICKET BOOTH SECTION AT GLAZING

T-S1.0 SCALE: 3/4" = 1'-0"



2 TICKET BOOTH SECTION AT SHEARWALLS

T-S1.0 SCALE: 3/4" = 1'-0"



3 TICKET BOOTH SECTION AT BACK SHEARWALL

T-S1.0 SCALE: 3/4" = 1'-0"

CONSTRUCTION DOCUMENTS

0 1/2" 1"

SCALE SHOWN TO ENSURE REPRODUCTION ACCURACY

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PROJECT NUMBER: 2338.0

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REVISIONS		
#	Description	Date

COMM. NO.	2012.030.00
MADE BY: HRG	CHECKED
DATE	5/30/12

TICKET BOOTH FOUNDATION & FRAMING PLANS

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